

## TECHNICAL NOTE

Date: 22 September 2021

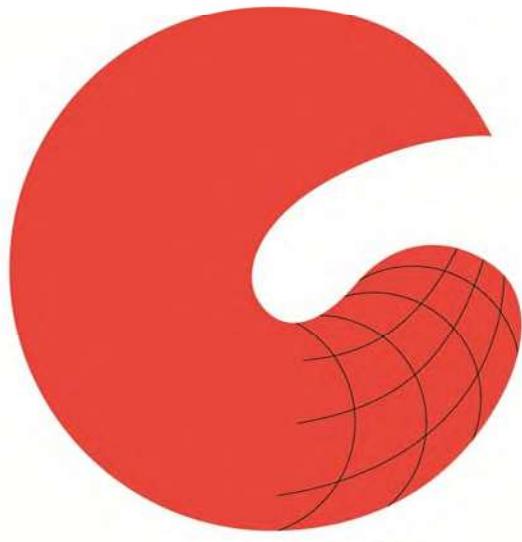
File Ref: SG/VL/P21-2315/01TN Vol. 2 of 3

Project: Land at Impington, Histon, Cambridgeshire

Subject: Summary of Flood Risk and Drainage Matters

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## **APPENDIX D**



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Watercourse Hydraulic Modelling Report

# NIAB, PARK FARM, CAMBRIDGE

## Watercourse Hydraulic Modelling Report

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**Reference:** JJ/CS/P17-1210/01

**Date:** March 2017

NIAB, PARK FARM, CAMBRIDGE

Watercourse Hydraulic Modelling Report

# **NIAB, PARK FARM, CAMBRIDGE**

## **Watercourse Hydraulic Modelling Report**

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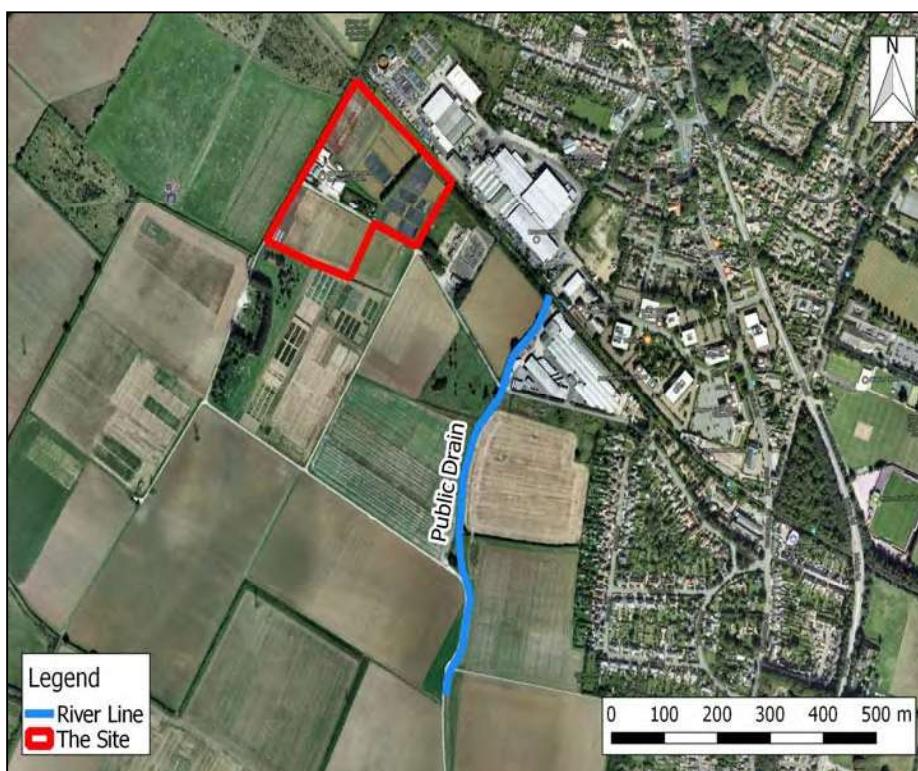
### **Registration of Amendments**

Revision and Date	Amendment Details	Revision Prepared By	Revision Approved By

## 1.0 INTRODUCTION

### Brief

- 1.1 This report provides a summary of the flood modelling assessment that was carried out by Create Consulting Engineers Ltd on behalf of the National Institute of Agricultural Botany (NIAB) Trust for the proposed development at Park Farm, 1 Villa Rd, Histon, Cambridge, CB24 9NZ.
- 1.2 The unnamed watercourse labelled on OS plans as 'Public Drain', located approximately 260 m to the south east of the site, was modelled using HEC-RAS software. The Public Drain and the site location is shown on Figure 1.1.



**Figure 1.1: Location of the Site and Public Drain**

### Project Context

- 1.3 The purpose of this report is to respond to the Environment Agency's (EA) request for a site specific hydraulic analysis of the watercourse (Public Drain) adjacent to Park Farm. This is to determine the flood risk to the proposed development with an allowance for climate change.
- 1.4 We understand that this hydraulic assessment will be submitted as part of a site specific Flood Risk Assessment for a planning application for development of a new glasshouse, two agricultural style buildings, laboratories, office space, and an additional carpark at Park Farm.

- 1.5 Architect's Layouts are included at the end of this report.
- 1.6 The Topographic Survey, is included within this report on Drawings ALS75618/OVERVIEW and ALS75618/500/01 to ALS75618/500/04. Relative to ordnance datum, ground levels at the site fall from 14.94 mAOD in the western extent of the site to 11.83 mAOD in the eastern extent of the site. It is to be noted that over the area that is proposed to be developed the ground levels range from 13.22 mAOD to 14.26 mAOD.

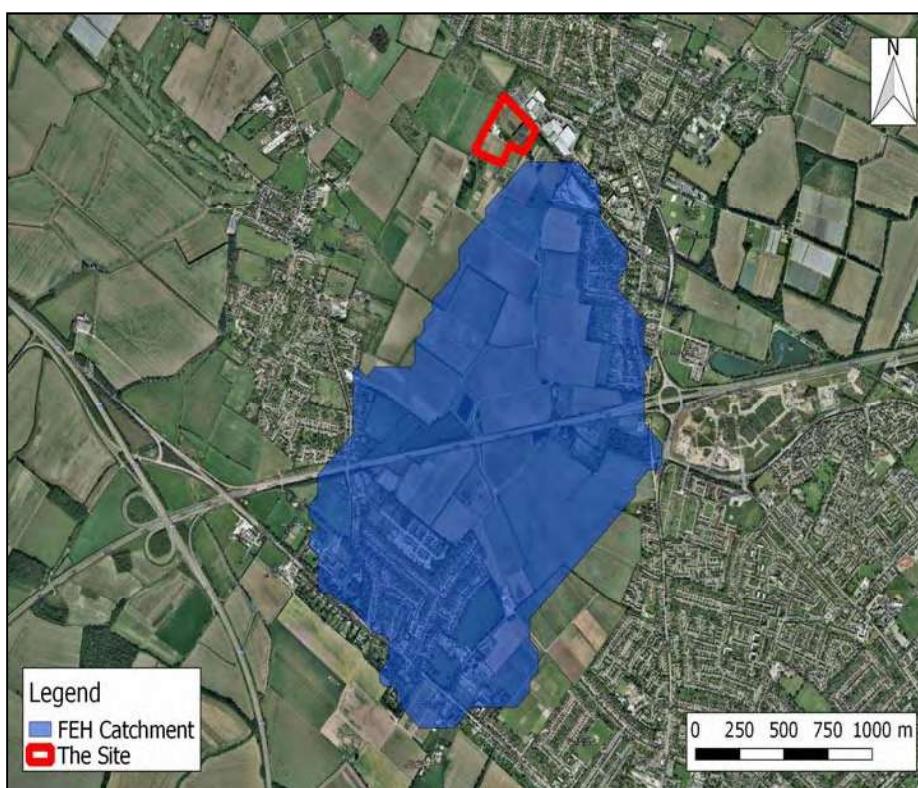
### **Constraints and Limitations**

- 1.7 The copyright of this report is vested in Create Consulting Engineers Ltd and the Client, NIAB Estate. The Client, or his appointed representatives, may copy the report for purposes in connection with the development described herein. It shall not be copied by any other party or used for any other purposes without the written consent of Create Consulting Engineers Ltd or the Client.
- 1.8 Create Consulting Engineers Ltd accepts no responsibility whatsoever to other parties to whom this report, or any part thereof, is made known. Any such other parties rely upon the report at their own risk.
- 1.9 The Watercourse Hydraulic Modelling Report addresses the flood risk posed to the proposed development, the extent of which is shown by the site boundary, as indicated by the location plan attached with this report.
- 1.10 This report has been undertaken with the assumption that the site will be developed in accordance with the above proposals without significant change. The conclusions resulting from this study are not necessarily indicative of future conditions or operating practices at or adjacent to the site.
- 1.11 Create Consulting Engineers Ltd has endeavored to assess all information provided to them during this appraisal. The report summarises information from a number of external sources and cannot offer any guarantees or warranties for the completeness or accuracy or information relied upon. Information from third parties has not been verified by Create Consulting Engineers Ltd unless otherwise stated in this report.

## 2.0 HYDROLOGICAL ASSESSMENT

### Watercourse and Catchment Area

- 2.1 The Public Drain drains towards the north combining with various other drains before reaching its confluence with the Beck Brook approximately 2 km to the north west of the site.
- 2.2 The area surrounding the drain is relatively flat and primarily comprises agricultural fields. The watercourse is understood to be fed by a catchment 3.27 km<sup>2</sup> in size to the south of the site. The catchment has been inferred from the FEH Web Mapping Service and verified against a watershed analysis using GRASS software and the Environment Agency LiDAR (Figure 2.1).



**Figure 2.1: Location of the FEH Catchment**

### Estimate of Flows in Watercourse

- 2.3 There is no gauged flow data available for the Public Drain and therefore the flow regime has been estimated using both WINFAP 4 and ReFH2 with catchment descriptors extracted from the FEH Web Mapping Service (Appendix A).
- 2.4 The median flow (QMED) for the study catchment (index flood) was calculated as 0.281 m<sup>3</sup>/s from the catchment descriptors using WINFAP 4.
- 2.5 Peak flow estimates produced by ReFH2 and WINFAP 4 are outlined below.

ReFH2

- 2.6 Using the ReFH2 program and the inputs from the FEH online web service (Appendix A), estimates were obtained for the site for various return periods.
- 2.7 The results are displayed in Table 2.1.

WINFAP 4

- 2.8 WINFAP 4 uses flow records from either a single reliable gauged site in the same catchment or from a number of other gauged sites in hydrologically similar catchments to form a pooling group which is subjected to a statistical analysis. Following this the flood growth curve and flood frequency curve generated by the analysis are used to calculate a range of flood flows at the site in question. This was carried out for the watercourse using the inputs from the FEH online web service (Appendix A). A default urban adjustment factor was applied (Urbext2000). The results are displayed in Table 2.1
- 2.9 A sensitivity analysis of the urban adjustment (URBext2000) factor was undertaken. The percentage of impermeable area was determined using aerial imagery and it was concluded that the percentage of impermeable area was approximately half of the default value presented by Urbext2000. The default Urbext2000 value was utilised for calculating flows as this value was more conservative than the calculated value.

Estimate of Bankfull Flow

- 2.10 A sensitivity analysis was carried out to determine the impact of using a bankfull flow estimate for QMED. WINFAP 4 has the functionality to produce a bankfull estimate for QMED.
- 2.11 The bankfull QMED was calculated using both the catchment descriptors and channel dimensions. The channel was measured in several places and an average Bankfull Channel Width of 5.95 was used. The results are displayed in Table 2.1.

Comparison of Flow Results

- 2.12 The results from each method are shown in Table 2.1.

Return Period (Years)	Flows Calculated using ReFH2 (m <sup>3</sup> /s)	Flows Calculated using WINFAP 4 (m <sup>3</sup> /s)	Flows Calculated from Bankfull Analysis (in WINFAP 4) (m <sup>3</sup> /s)
2	0.49	0.28	0.63
5	Not Modelled	0.39	0.87
10	Not Modelled	0.48	1.06

Return Period (Years)	Flows Calculated using ReFH2 (m <sup>3</sup> /s)	Flows Calculated using WINFAP 4 (m <sup>3</sup> /s)	Flows Calculated from Bankfull Analysis (in WINFAP 4) (m <sup>3</sup> /s)
20	0.89	Not Modelled	Not Modelled
25	Not Modelled	0.60	1.33
50	Not Modelled	0.70	1.57
100	1.34	0.83	1.84
200	Not Modelled	0.97	2.16
500	Not Modelled	1.19	2.65
1000	2.51	1.39	3.10

**Table 2.1: Flows Calculated using 1) ReFH2, 2) WINFAP 4, and 3) Bankfull Analysis (Using WINFAP) for Several Return Periods**

- 2.13 To provide a conservative estimate of the flows in the Public Drain, peak flows from the bankfull analysis will be utilised in the flood model.

#### Climate Change Allowances

- 2.14 To incorporate the potential future effects of climate change the bankfull flows have been increased accordingly.
- 2.15 The development is classified as ‘less vulnerable’ use according to the NPPF. Based on the EA’s<sup>1</sup> guidance the ‘central’ (25%) and ‘higher central’ (35%) allowances for the Anglian Basin 2070-2115 will be added on to the flows based on a development design life of 100 years. Correspondence with the Environment Agency (28th February to 13th March 2017) confirmed that these allowances are acceptable. The results are displayed in Table 2.1.

Return Period (Years)	Flows Calculated from Bankfull Analysis (in WINFAP 4)	+25% Climate Change	+35% Climate Change
2	0.63	0.78	0.85
5	0.87	1.09	1.18
10	1.06	1.32	1.43
25	1.33	1.66	1.79
50	1.57	1.96	2.11
100	1.84	2.30	2.48
200	2.16	2.69	2.91
500	2.65	3.31	3.58
1000	3.10	3.87	4.18

**Table 2.2: Flows calculated using bankfull analysis (in WINFAP) to include 25% climate change and 35% climate change allowances**

<sup>1</sup> Environment Agency (2016) *Flood Risk Assessments: Climate Change Allowances*.

### 3.0 METHODOLOGY

- 3.1 The hydraulic model has been developed to estimate the flood extent of the Public Drain for the 1 in 100 year event. As the channel is both small and relatively straight a one dimensional model was chosen to represent the channel which has been constructed using HEC-RAS 5.0. A 700m stretch of the drain (identified in Figure 1.1) ending at the Bypass was chosen to best represent water levels at the site. This reach comprises 4 bridges and one culvert.
- 3.2 The model was run in a steady state scenario by using a constant flow input for the 1 in 100 year event (as the pooling analysis generates a peak flow only).

#### **Channel Survey Data**

- 3.3 A detailed topographical survey was undertaken on 7 March 2017 (Drawings 2219-545-SU01 - 2219-545-SU16). 42 channel cross sections at 20 m intervals were generated using the topographical survey as well as a 3D surface.

#### **Model Schematic**

- 3.4 A schematic representation of the model is included in Figure 3.1 at the rear of the report.

#### **Surface Roughness**

##### Channel and Floodplain

- 3.5 The following surface roughness assumptions have been made for the channel and floodplain based on a combination of site photographs and google earth imagery. The values have been obtained from the HEC-RAS reference manual (US Army Corps Engineers, 2016). The Manning's 'n' values are shown in Table 3.1 below.

Geometry	Manning's 'n'	Description
Channel	0.35	Clean, straight, full, no rifts or deep pools but more stones and weeds
Floodplain	0.04	Mature field crops

**Table 3.1: Manning's 'n' values used for the channel and floodplain in HEC-RAS**

##### Structures

- 3.6 The four bridges have been modelled using the dimensions, soffit levels, and invert levels provided within the topographical survey. The bridges are clear span with no piers or edges so it was assumed the surface roughness would remain the same as the channel. The culvert located in the northern part of the model has been modelled as an arched culvert. The

following surface roughness assumptions have been made and the Manning's 'n' values are shown in Table 3.2 below.

Geometry	Manning's 'n'	Description
Bridges	0.35	Remained the same as channel
Culvert	0.015	Culvert with some debris

**Table 3.2: Manning's 'n' values used for the channel and floodplain in HEC-RAS**

### **Contraction and Expansion**

- 3.7 Contraction and expansion coefficients for the bridges and culverts were obtained from the HEC-RAS manual as shown in Table 3.3 below.

Geometry	Contraction Coefficient	Expansion Coefficient	Description
Bridges	0.3	0.5	Typical bridge
Culvert	0.6	0.8	Abrupt transition

**Table 3.3: Contraction and expansion values used in HEC-RAS**

### **Boundary Conditions**

- 3.8 A bed slope of 0.0008 was used throughout the length of the model, which was calculated (from the topographic survey) as the average gradient of the channel along the section in question.

## 4.0 RESULTS OF MODELLING

- 4.1 The 1 in 100 year event with an allowance for climate change was modelled and the results are shown in Table 4.1 (to the rear of the report). The 100 year with a 35% allowance for climate change scenario has been selected as the design flood event for the development when assessing the flood risk to the development with additional considerations made to the results of sensitivity scenarios carried out.
- 4.2 For the 100 year plus 35% climate change scenario, levels in the channel ranged from 10.87m AOD to 11.63m AOD. Freeboard along the left bank ranges from 10 mm to 590 mm and along the right bank it ranges from 110 mm to 550 mm.
- 4.3 In the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.87m AOD to 11.23m AOD. Freeboard along the left bank ranges from 40 mm to 590 mm and along the right bank it ranges from 110 mm to 550 mm.
- 4.4 The results show that flow remains in channel along the modelled reach (as shown in the long profile Figure 4.1). The lowest levels of the site are between 12.09 m AOD and 11.84 m AOD in the eastern corner. It is to be noted that over the area that is proposed to be developed the ground levels range from 13.22 mAOD to 14.26 mAOD.
- 4.5 The site levels are well above the design flood event peak level of 11.23 mAOD with the proposed developed area (13.22 mAOD) being 1.99m above this level. It is concluded that the development is at a low risk of flooding from the public drain and has sufficient freeboard above the design flood event (100yr + cc) for the lifetime of the development.

### Sensitivity Testing

- 4.6 As requested by the EA several sensitivity tests were undertaken to test the assumptions made within the model. These included an uplift of the following:
  - Coefficients used for Manning's 'n' hydraulic roughness
  - Contraction/expansion losses
  - Peak flows
- 4.7 The results are discussed in the following sections.

#### Manning's 'n'

- 4.8 A sensitivity test was undertaken using the modelled design flood event (1 in 100 year flow plus 35% climate change) with a 25% increase in the channel and floodplain Manning's 'n' values. This increased the Mannings 'n' values to 0.044 for the channel and 0.05 for the floodplain. Results are displayed in Table 4.2, to the rear of the report.

- 4.9 In the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.99m AOD to 11.36m AOD. Freeboard along the left bank ranges from -80 mm (exceeding bank level) to 470 mm and along the right bank it ranges from -10 mm to 420 mm.
- 4.10 The results show that the flows primarily remain in channel, however, it overtops within chainage 177 to 0 in one location along the left bank (depth of 80 mm) and one along the right bank (depth of 10 mm).
- 4.11 The lowest levels of the site are between 12.09 m AOD and 11.84 m AOD (in the eastern corner). The site will therefore remain dry in this event. Additionally the lowest part of the site with development proposals is at 13.22 m AOD (Barn 03).
- 4.12 The site levels are well above the design flood event peak level of 11.36 mAOD with the proposed developed area (13.22 mAOD) being 1.86m above this level. It is concluded that with a 25% increase in Manning's n, the development remains at a low risk of flooding for the design flood event.

#### Contraction/Expansion Losses

- 4.13 A sensitivity test was undertaken using the modelled design flood event with a 25% increase in the contraction and expansion losses values for the bridges and culvert. Results are displayed in Table 4.3, to the rear of the report
- 4.14 In the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.87m AOD to 11.23m AOD. Freeboard along the left bank ranges from 40 mm to 580 mm and along the right bank it ranges from 110 mm to 550 mm.
- 4.15 The results show that the flows remain in channel in all locations within chainage 177-0. Additionally, the lowest levels of the site are between 12.09 m AOD and 11.84 m AOD (in the eastern corner). The site therefore remains dry during this event.
- 4.16 The site levels are well above the design flood event peak level of 11.23 mAOD with the proposed developed area (13.22 mAOD) being 1.99m above this level. It is concluded that with a 25% increase in contraction and expansion losses, the development remains at a low risk of flooding for the design flood event.

#### Peak Flows

- 4.17 A sensitivity test was undertaken using the design flood event with a 25% increase to the peak flow (roughly equivalent to the 1000yr event). Results are displayed in Table 4.4, to the rear of the report.

- 4.18 In the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.99m AOD to 11.35m AOD. Freeboard along the left bank ranges from -80 mm (exceeding bank level) to 480 mm and along the right bank it ranges from -10 mm to 430 mm.
- 4.19 The results show that the flows remain in channel in all locations within chainage 177-0. Additionally, the lowest levels of the site are between 12.09 m AOD and 11.84 m AOD (in the eastern corner). The site therefore remains dry during this event.
- 4.20 The site levels are well above the design flood event peak level of 11.35 mAOD with the proposed developed area being 1.87m above this level. It is concluded that with a 25% increase in peak flow, the development remains at a low risk of flooding for the design flood event.

### Calibration

- 4.21 There is no gauged data or data from observed events available for calibration purposes.
- 4.22 It is noted however that the EA Surface Water Flood Maps (Figure 3.1) shows the site to be at 'very low' flood risk. This provides a form of validation against the results, showing that the site is not affected.

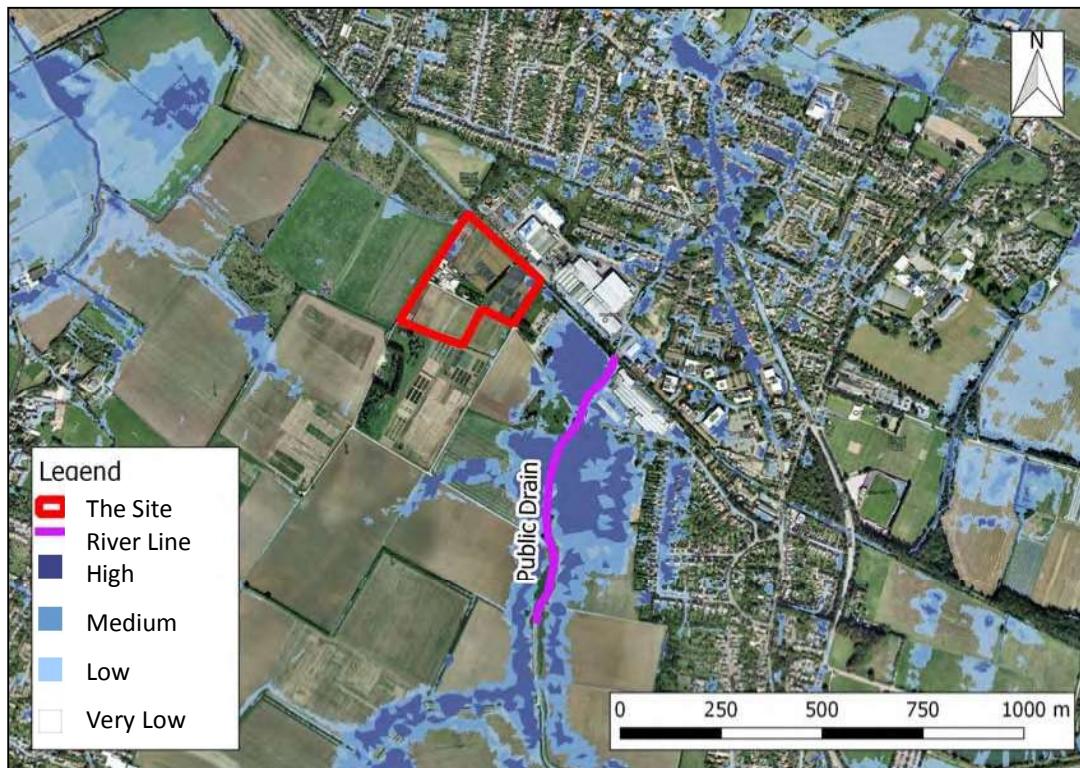


Figure 4.2 EA Surface Water Flood Maps

## 5.0 CONCLUSIONS

- 5.1 This report has provided a summary of the flood modelling assessment that was carried out by Create Consulting Engineers Ltd on behalf of the NIAB Trust for the proposed development at Park Farm, 1 Villa Rd, Histon, Cambridge, CB24 9NZ.
- 5.2 A hydrological assessment was undertaken to calculate the flows for the Public Drain. The bankfull estimate (using WINFAP 4) was used as it was the most conservative estimate, with an addition of 25% and 35% for climate change.
- 5.3 The hydraulic model has been developed to estimate the flood risk to the development from the Public Drain for the 1 in 100 year event with an allowance for climate change. As the channel is both small and relatively straight a one dimensional model was chosen to represent the channel which has been constructed using HEC-RAS 5.0.
- 5.4 The model was based on a detailed topographical survey was undertaken on 7th March 2017 (Drawings 2219-545-SU01 - 2219-545-SU16). 42 channel cross sections at 20 m intervals were generated using the topographical survey as well as a 3D surface.
- 5.5 The results of the model for the 100yr +35% flood event (the chosen design flood event) showed that the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.87m AOD to 11.23m AOD. Freeboard along the left bank ranges from 40 mm to 590 mm and along the right bank it ranges from 110 mm to 550 mm.
- 5.6 The results showed that flow remains in channel along the modelled reach. The site levels are well above the design flood event peak level of 11.23 mAOD with the proposed developed area (13.22 mAOD) being 1.99m above this level.
- 5.7 As requested by the EA several sensitivity tests were undertaken to test the assumptions made within the model. These included an uplift of the following:
  - Coefficients used for Manning's 'n' hydraulic roughness
  - Contraction/expansion losses
  - Peak flows
- 5.8 The results of these sensitivity tests shows the site to not be affected by any of the flood events.
- 5.9 There is no gauged data or data from observed events available for calibration purposes. It is noted, however, that the EA Surface Water Flood Maps shows the site to be at 'very low' flood risk. This provides a form of validation against the results, showing that the site is not affected.

- 5.10 It is concluded that the levels on site are situated sufficiently above flood levels modelled in the public drain. It also concluded that the development will be at a low risk of flooding from the public drain and will have sufficient freeboard above the design flood event (100yr + cc) for the lifetime of the development.

## 6.0 REFERENCES

- i. Environment Agency Surface Water Flood WMS Layers (2014) Available at: <https://data.gov.uk/data/search> (Accessed March 2017).
- ii. US Army Corps Engineers (2016). HEC-RAS River Analysis System User's Manual. Version 5.0.

## **FIGURES**

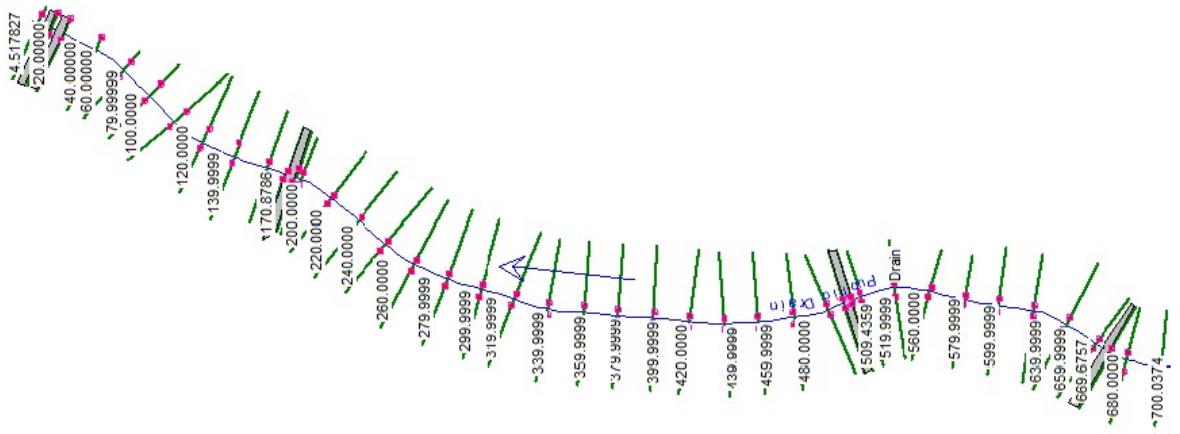


Figure 3.1 Model Schematic Exported from HEC-RAS

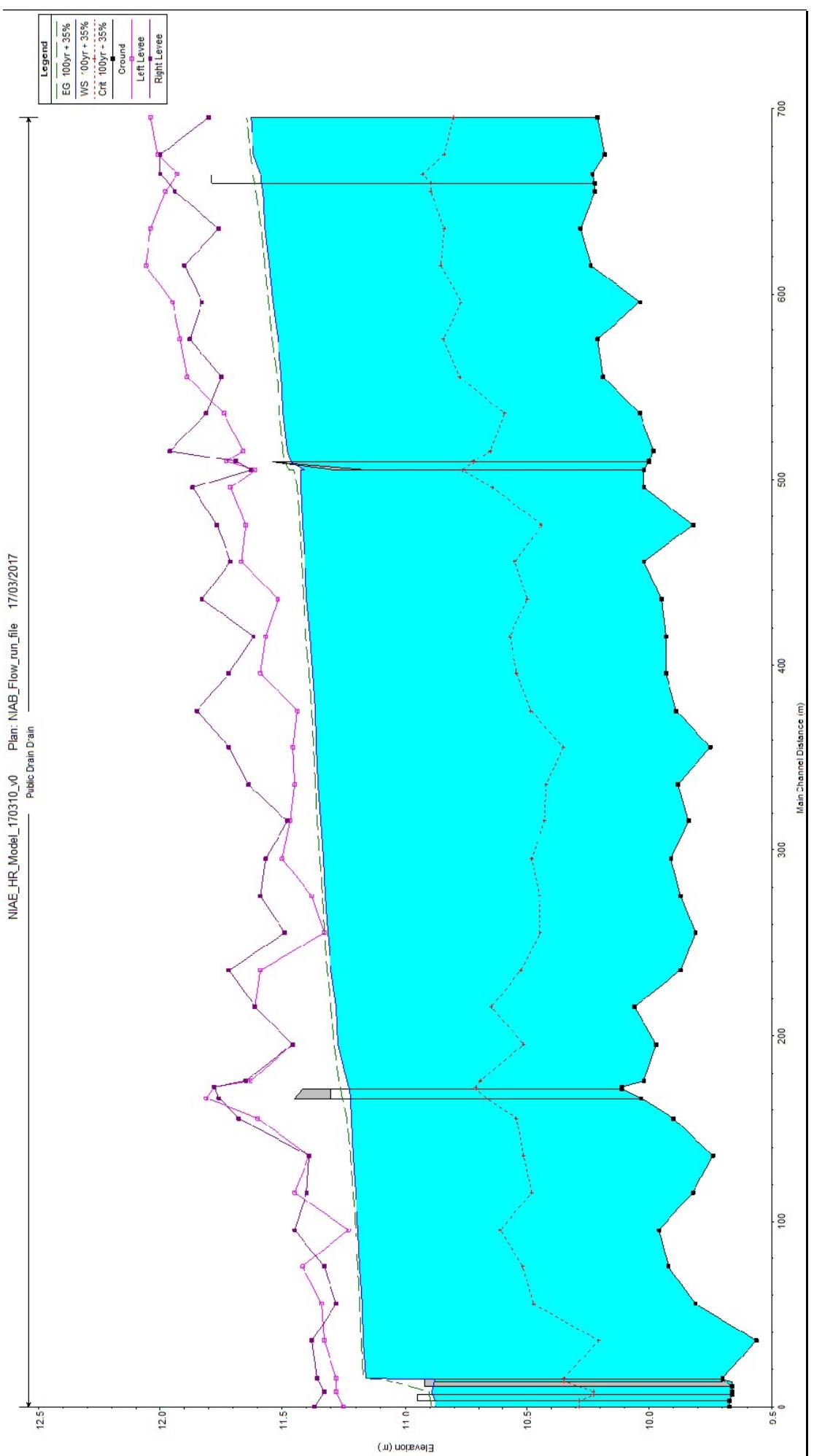


Figure 4.1: Long Profile of 100 year +35% Climate Change Model Run

## **TABLES**

Reach	River Station	Profile	Left Freeboard (m)	Right Freeboard (m)	Water Surface Elevation (m)	Critical Water Surface (m)	Energy Grade Elevation (m)	Energy Grade Slope (m/m)	Velocity Channel (m/s)	Flow Area (m <sup>2</sup> )	Top Width (m)	Froude Number Channel
Drain	700.0374	100yr + 35%	0.41	0.17	11.63	10.8	11.65	0.000524	0.54	4.6	5.2	0.18
Drain	680.100yr + 35%	0.39	0.38	11.62	10.84	11.63	0.000619	0.57	4.33	5.11	0.2	0.29
Drain	669.6757	100yr + 35%	0.34	0.41	11.59	10.93	11.62	0.001524	0.81	3.06	3.8	0.29
Drain	664.9016	None	Bridge									
Drain	659.9999	100yr + 35%	0.4	0.36	11.58	10.89	11.61	0.001097	0.74	3.34	3.69	0.25
Drain	639.9999	100yr + 35%	0.47	0.19	11.57	10.84	11.59	0.000511	0.53	4.65	5.45	0.18
Drain	620.100yr + 35%	0.51	0.35	11.55	10.85	11.57	0.000775	0.63	3.95	4.8	0.22	
Drain	599.9999	100yr + 35%	0.41	0.29	11.54	10.77	11.56	0.000658	0.6	4.16	4.67	0.2
Drain	579.9999	100yr + 35%	0.4	0.36	11.52	10.84	11.54	0.000985	0.68	3.62	4.53	0.24
Drain	560.100yr + 35%	0.39	0.25	11.5	10.78	11.52	0.00079	0.64	3.86	4.44	0.22	
Drain	540.100yr + 35%	0.24	0.31	11.5	10.59	11.51	0.000456	0.51	4.83	5.23	0.17	
Drain	519.9999	100yr + 35%	0.18	0.48	11.48	10.65	11.5	0.000714	0.62	4.03	4.4	0.21
Drain	514.1835	100yr + 35%	0.26	0.22	11.47	10.72	11.49	0.001125	0.73	3.4	3.93	0.25
Drain	511.7022	None	Bridge									
Drain	509.4359	100yr + 35%	0.18	0.2	11.43	10.76	11.46	0.001282	0.75	3.29	4.21	0.27
Drain	499.9999	100yr + 35%	0.29	0.45	11.42	10.64	11.44	0.000601	0.58	4.29	4.79	0.19
Drain	480.100yr + 35%	0.23	0.35	11.42	10.44	11.43	0.000341	0.46	5.34	5.35	0.15	
Drain	459.9999	100yr + 35%	0.26	0.3	11.41	10.55	11.42	0.000461	0.52	4.8	5.27	0.17
Drain	439.9999	100yr + 35%	0.12	0.43	11.4	10.5	11.41	0.000463	0.52	4.78	5.11	0.17
Drain	420.100yr + 35%	0.18	0.23	11.39	10.57	11.4	0.000566	0.56	4.41	4.84	0.19	
Drain	399.9999	100yr + 35%	0.21	0.34	11.38	10.54	11.39	0.000559	0.55	4.48	5.11	0.19
Drain	379.9999	100yr + 35%	0.07	0.48	11.37	10.49	11.38	0.000478	0.52	4.74	5.21	0.17
Drain	359.9999	100yr + 35%	0.1	0.36	11.36	10.35	11.37	0.000289	0.42	5.84	6.13	0.14
Drain	339.9999	100yr + 35%	0.1	0.29	11.35	10.42	11.37	0.000384	0.48	5.19	5.55	0.16
Drain	319.9999	100yr + 35%	0.13	0.14	11.34	10.43	11.36	0.000442	0.49	5.05	5.88	0.17
Drain	299.9999	100yr + 35%	0.17	0.23	11.33	10.48	11.35	0.000471	0.51	4.9	5.77	0.18
Drain	279.9999	100yr + 35%	0.05	0.26	11.33	10.45	11.34	0.000446	0.5	4.99	5.91	0.17
Drain	260.100yr + 35%	0.01	0.17	11.32	10.44	11.33	0.000519	0.5	4.93	6.39	0.18	
Drain	240.100yr + 35%	0.29	0.42	11.3	10.52	11.32	0.00055	0.55	4.5	5.2	0.19	
Drain	220.100yr + 35%	0.33	0.33	11.28	10.64	11.3	0.000995	0.67	3.69	4.97	0.25	
Drain	200.100yr + 35%	0.19	0.19	11.27	10.51	11.29	0.000533	0.54	4.6	5.37	0.19	
Drain	179.9999	100yr + 35%	0.39	0.41	11.24	10.69	11.27	0.001368	0.78	3.19	4.29	0.29
Drain	176.7426	100yr + 35%	0.55	0.55	11.23	10.71	11.26	0.001542	0.81	3.04	4.16	0.3
Drain	173.7661	None	Bridge									
Drain	170.8786	100yr + 35%	0.59	0.54	11.22	10.67	11.25	0.001353	0.78	3.17	4.13	0.29
Drain	159.9999	100yr + 35%	0.38	0.46	11.22	10.54	11.24	0.000717	0.59	4.18	5.45	0.22
Drain	139.9999	100yr + 35%	0.19	0.19	11.21	10.51	11.22	0.000581	0.45	5.46	9.54	0.19
Drain	120.100yr + 35%	0.26	0.2	11.2	10.48	11.21	0.000456	0.42	5.84	9.51	0.17	
Drain	100.100yr + 35%	0.04	0.27	11.19	10.61	11.2	0.000564	0.45	5.52	9.85	0.19	
Drain	79.9999	100yr + 35%	0.25	0.15	11.18	10.52	11.19	0.00038	0.39	6.38	10.48	0.16
Drain	60.100yr + 35%	0.16	0.11	11.17	10.47	11.18	0.000424	0.42	5.9	9.29	0.17	
Drain	40.100yr + 35%	0.16	0.21	11.17	10.21	11.18	0.000194	0.32	7.8	10.34	0.12	
Drain	20.100yr + 35%	0.12	0.2	11.16	10.35	11.17	0.000638	0.47	5.32	9.47	0.2	
Drain	19.05487	None	Culvert									
Drain	12.28586	100yr + 35%	0.39	0.44	10.89	10.22	10.9	0.000673	0.51	4.9	8.15	0.21
Drain	7.988860	None	Bridge		0.5	10.87	10.28	0.001	0.63	3.93	6.2	0.25

Table 4.1:1 in 100 Year + 35% Climate Change

Reach	River Station	Profile	Left Freeboard (m)	Right Freeboard (m)	Water Surface Elevation (m)	Critical Water Surface (m)	Energy Grade Elevation (m)	Energy Grade Slope (m/m)	Velocity Channel (m/s)	Flow Area (m <sup>2</sup> )	Top Width (m)	Froude Number Channel
Drain	700.0374	100yr + 35%	0.27	0.03	11.77	10.8	11.78	0.000578	0.46	5.35	5.83	0.15
Drain	680.100yr + 35%		0.26	0.25	11.75	10.84	11.77	0.000649	0.49	5.06	5.51	0.16
Drain	669.6757	100yr + 35%	0.2	0.27	11.73	10.93	11.76	0.001561	0.68	3.63	4.25	0.24
Drain	664.9016	None	Bridge									
Drain	659.9999	100yr + 35%	0.26	0.22	11.72	10.89	11.74	0.001165	0.64	3.88	3.91	0.21
Drain	639.9999	100yr + 35%	0.33	0.05	11.71	10.84	11.72	0.000528	0.46	5.43	5.77	0.15
Drain	620.100yr + 35%		0.37	0.21	11.69	10.85	11.71	0.000795	0.53	4.64	5.2	0.18
Drain	599.9999	100yr + 35%	0.27	0.15	11.68	10.77	11.69	0.000728	0.51	4.83	5.25	0.17
Drain	579.9999	100yr + 35%	0.26	0.22	11.66	10.84	11.68	0.00102	0.58	4.29	5.07	0.2
Drain	560.100yr + 35%		0.25	0.11	11.64	10.78	11.66	0.000886	0.55	4.5	5.09	0.19
Drain	540.100yr + 35%		0.11	0.18	11.63	10.59	11.64	0.000499	0.45	5.56	5.64	0.14
Drain	519.9999	100yr + 35%	0.05	0.35	11.61	10.65	11.63	0.00078	0.53	4.64	4.77	0.17
Drain	514.1835	100yr + 35%	0.13	0.09	11.16	10.72	11.62	0.001247	0.62	3.98	4.55	0.21
Drain	511.7022	None	Bridge									
Drain	509.4359	100yr + 35%	0.07	0.09	11.54	10.76	11.57	0.001421	0.65	3.82	4.72	0.23
Drain	499.9999	100yr + 35%	0.17	0.33	11.54	10.64	11.55	0.000692	0.51	4.86	5.16	0.17
Drain	480.100yr + 35%		0.12	0.24	11.53	10.44	11.54	0.000427	0.41	5.98	6.03	0.13
Drain	459.9999	100yr + 35%	0.15	0.19	11.52	10.55	11.53	0.000532	0.46	5.41	5.58	0.15
Drain	439.9999	100yr + 35%	0.01	0.32	11.51	10.5	11.52	0.000545	0.46	5.36	5.45	0.15
Drain	420.100yr + 35%		0.07	0.12	11.15	10.57	11.51	0.00069	0.5	4.96	5.41	0.17
Drain	399.9999	100yr + 35%	0.11	0.24	11.48	10.54	11.49	0.000651	0.49	5.04	5.45	0.16
Drain	379.9999	100yr + 35%	-0.04	0.37	11.48	10.49	11.49	0.001051	0.25	16.18	42.49	0.08
Drain	359.9999	100yr + 35%	-0.02	0.24	11.48	10.35	11.48	0.00114	0.22	17.53	42.81	0.07
Drain	339.9999	100yr + 35%	-0.03	0.16	11.48	10.42	11.48	0.00174	0.27	14.77	42.88	0.09
Drain	319.9999	100yr + 35%	0.01	0.02	11.46	10.43	11.47	0.000531	0.43	5.81	6.92	0.15
Drain	299.9999	100yr + 35%	0.05	0.12	11.45	10.48	11.46	0.000545	0.44	5.62	6.51	0.15
Drain	279.9999	100yr + 35%	-0.07	0.14	11.45	10.45	11.45	0.00143	0.23	16.66	42.65	0.08
Drain	260.100yr + 35%		-0.12	0.04	11.45	10.44	11.45	0.00161	0.24	15.93	42.89	0.08
Drain	240.100yr + 35%		0.16	0.29	11.43	10.52	11.44	0.000588	0.48	5.2	5.53	0.16
Drain	220.100yr + 35%		0.2	0.2	11.41	10.64	11.43	0.001254	0.56	4.42	6.77	0.22
Drain	200.100yr + 35%		0.06	0.06	11.14	10.51	11.41	0.00058	0.47	5.31	5.79	0.16
Drain	179.9999	100yr + 35%	0.26	0.28	11.37	10.69	11.39	0.00137	0.66	3.78	4.62	0.23
Drain	176.7426	100yr + 35%	0.42	0.42	11.36	10.71	11.39	0.015385	0.68	3.63	4.74	0.25
Drain	173.7661	None	Bridge									
Drain	170.8786	100yr + 35%	0.47	0.42	11.34	10.67	11.36	0.001448	0.68	3.66	4.39	0.24
Drain	159.9999	100yr + 35%	0.27	0.35	11.33	10.54	11.34	0.000792	0.51	4.83	5.98	0.18
Drain	139.9999	100yr + 35%	0.08	0.08	11.32	10.51	11.33	0.000564	0.38	6.58	10.61	0.15
Drain	120.100yr + 35%		0.15	0.09	11.31	10.48	11.32	0.000474	0.36	6.97	10.86	0.14
Drain	100.100yr + 35%		-0.08	0.15	11.31	10.61	11.31	0.00216	0.24	14.26	40.75	0.1
Drain	79.9999	100yr + 35%	0.13	0.03	11.3	10.52	11.3	0.00037	0.32	7.66	11.54	0.13
Drain	60.100yr + 35%		0.04	-0.01	11.29	10.47	11.3	0.00044	0.35	7.06	10.85	0.14
Drain	40.100yr + 35%		0.04	0.09	11.29	10.21	11.29	0.00211	0.27	9.05	11.32	0.1
Drain	20.100yr + 35%		0	0.08	11.28	10.35	11.28	0.00296	0.25	13.27	43.1	0.11
Drain	19.05487	None	Culvert									
Drain	12.28586	100yr + 35%	0.27	0.32	11.01	10.22	11.02	0.000652	0.41	5.98	9.35	0.17
Drain	7.988260	None	Bridge									
Drain	4.517827	100yr + 35%	0.26	0.38	10.99	10.28	11	0.01002	0.53	4.7	6.88	0.2

Table 4.2.1 in 100 Year + 35% Climate Change Plus 25% Increase in Mannings

Reach	River Station	Profile	Left Freeboard (m)	Right Freeboard (m)	Water Surface Elevation (m)	Critical Water Surface (m)	Energy Grade Elevation (m)	Energy Slope (m/m)	Velocity Chanee (m/s)	Flow Area (m <sup>2</sup> )	Top Width (m)	Froude Number Chanel
Drain	700.0374	100yr + 35%	0.4	0.16	11.64	10.8	11.66	0.000509	0.53	4.66	5.25	0.18
Drain	680.100yr + 35%	0.38	0.37	11.63	10.84	11.65	0.000597	0.56	4.39	5.14	0.2	
Drain	669.6757 100yr + 35%	0.33	0.4	11.6	10.93	11.63	0.001466	0.8	3.1	3.84	0.28	
Drain	664.9016 None	Bridge										
Drain	659.9999 100yr + 35%	0.39	0.35	11.59	10.89	11.62	0.001059	0.73	3.39	3.71	0.24	
Drain	639.9999 100yr + 35%	0.46	0.18	11.58	10.84	11.6	0.000494	0.53	4.71	5.48	0.18	
Drain	620.100yr + 35%	0.49	0.33	11.57	10.85	11.58	0.000748	0.62	4	4.83	0.22	
Drain	599.9999 100yr + 35%	0.4	0.28	11.55	10.77	11.57	0.000636	0.59	4.21	4.7	0.2	
Drain	579.9999 100yr + 35%	0.39	0.35	11.53	10.84	11.55	0.000948	0.67	3.68	4.58	0.24	
Drain	560.100yr + 35%	0.37	0.23	11.52	10.78	11.54	0.00076	0.63	3.92	4.47	0.22	
Drain	540.100yr + 35%	0.23	0.3	11.51	10.59	11.52	0.00044	0.51	4.9	5.27	0.17	
Drain	519.9999 100yr + 35%	0.17	0.47	11.49	10.65	11.51	0.000689	0.61	4.08	4.44	0.2	
Drain	514.1835 100yr + 35%	0.25	0.21	11.48	10.72	11.5	0.001089	0.72	3.45	3.99	0.25	
Drain	511.7022 None	Bridge										
Drain	509.4359 100yr + 35%	0.18	0.2	11.43	10.76	11.46	0.00126	0.75	3.32	4.23	0.27	
Drain	499.9999 100yr + 35%	0.28	0.44	11.43	10.64	11.45	0.000594	0.57	4.31	4.8	0.19	
Drain	480.100yr + 35%	0.23	0.35	11.42	10.44	11.44	0.000338	0.46	5.36	5.36	0.15	
Drain	459.9999 100yr + 35%	0.26	0.3	11.41	10.55	11.43	0.000456	0.51	4.83	5.28	0.17	
Drain	439.9999 100yr + 35%	0.12	0.43	11.4	10.5	11.42	0.000458	0.52	4.8	5.12	0.17	
Drain	420.100yr + 35%	0.18	0.23	11.39	10.57	11.41	0.000561	0.56	4.43	4.86	0.19	
Drain	399.9999 100yr + 35%	0.21	0.34	11.38	10.54	11.4	0.000552	0.55	4.5	5.12	0.19	
Drain	379.9999 100yr + 35%	0.07	0.48	11.37	10.49	11.39	0.000472	0.52	4.77	5.22	0.17	
Drain	359.9999 100yr + 35%	0.09	0.35	11.37	10.35	11.38	0.000287	0.42	5.86	6.16	0.14	
Drain	339.9999 100yr + 35%	0.09	0.28	11.36	10.42	11.37	0.000379	0.48	5.21	5.56	0.16	
Drain	319.9999 100yr + 35%	0.12	0.13	11.35	10.43	11.36	0.000438	0.49	5.07	5.92	0.17	
Drain	299.9999 100yr + 35%	0.16	0.23	11.34	10.48	11.35	0.000466	0.5	4.93	5.8	0.17	
Drain	279.9999 100yr + 35%	0.05	0.26	11.33	10.45	11.34	0.000456	0.49	5.01	5.95	0.17	
Drain	260.100yr + 35%	0.01	0.17	11.32	10.44	11.33	0.000515	0.5	4.96	6.44	0.18	
Drain	240.100yr + 35%	0.28	0.41	11.31	10.52	11.32	0.000543	0.55	4.52	5.21	0.19	
Drain	220.100yr + 35%	0.33	0.33	11.28	10.64	11.31	0.000938	0.67	3.71	4.99	0.25	
Drain	200.100yr + 35%	0.18	0.18	11.28	10.51	11.29	0.000526	0.54	4.62	5.38	0.19	
Drain	179.9999 100yr + 35%	0.39	0.41	11.24	10.69	11.27	0.001348	0.77	3.21	4.3	0.29	
Drain	176.7426 100yr + 35%	0.55	0.55	11.23	10.71	11.27	0.00152	0.81	3.06	4.18	0.3	
Drain	173.7661 None	Bridge										
Drain	170.8786 100yr + 35%	0.58	0.53	11.23	10.67	11.26	0.001335	0.78	3.18	4.14	0.28	
Drain	159.9999 100yr + 35%	0.38	0.46	11.22	10.54	11.24	0.000712	0.59	4.19	5.46	0.22	
Drain	139.9999 100yr + 35%	0.19	0.19	11.21	10.51	11.22	0.000577	0.45	5.47	9.56	0.19	
Drain	120.100yr + 35%	0.26	0.2	11.2	10.48	11.21	0.000453	0.42	5.85	9.53	0.17	
Drain	100.100yr + 35%	0.04	0.27	11.19	10.61	11.2	0.00056	0.45	5.54	9.87	0.19	
Drain	79.9999 100yr + 35%	0.25	0.15	11.18	10.52	11.19	0.000378	0.39	6.39	10.5	0.16	
Drain	60.100yr + 35%	0.16	0.11	11.17	10.47	11.18	0.000421	0.42	5.91	9.3	0.17	
Drain	40.100yr + 35%	0.16	0.21	11.17	10.21	11.18	0.000194	0.32	7.81	10.35	0.12	
Drain	20.100yr + 35%	0.12	0.2	11.16	10.35	11.17	0.000635	0.47	5.33	9.49	0.2	
Drain	19.05487 None	Culvert										
Drain	12.228586 100yr + 35%	0.39	0.44	10.89	10.22	10.9	0.00067	0.51	4.91	8.16	0.21	
Drain	7.938860 None	Bridge										
Drain	4.517827 100yr + 35%	0.38	0.5	10.87	10.28	10.89	0.001	0.63	3.93	6.2	0.25	

Table 4.1: in 100 Year + 35% Climate Change Plus 25% Increase on Contraction and Expansion Losses

Reach	River Station	Profile	Left Freeboard (m)	Right Freeboard (m)	Water Surface Elevation (m)	Critical Water Surface (m)	Energy Grade Elevation (m)	Energy Grade Slope (m/m)	Velocity Channel (m/s)	Flow Area (m <sup>2</sup> )	Top Width (m)	Froude Number	Chanel	
Drain	700.03741	100yr + 35%	0.25	0.01	11.79	10.87	11.81	0.00054	0.57	5.48	5.93	0.19		
Drain	680.100yr + 35%	0.23	0.22	11.78	10.92	11.79	0.000603	0.6	5.17	5.57	0.2	0.2		
Drain	669.67571	100yr + 35%	0.18	0.25	11.75	11.02	11.78	0.001474	0.84	3.69	4.3	0.29		
Drain	664.9016	None	Bridge											
Drain	659.9999	100yr + 35%	0.24	0.2	11.74	10.97	11.77	0.001101	0.79	3.94	3.93	0.25		
Drain	639.9999	100yr + 35%	0.31	0.03	11.73	10.9	11.75	0.000492	0.56	5.54	5.82	0.18		
Drain	620.100yr + 35%	0.35	0.19	11.71	10.92	11.73	0.000743	0.65	4.74	5.25	0.22			
Drain	599.9999	100yr + 35%	0.25	0.13	11.7	10.84	11.72	0.000685	0.63	4.94	5.35	0.21		
Drain	579.9999	100yr + 35%	0.24	0.2	11.68	10.92	11.7	0.000951	0.71	4.39	5.15	0.24		
Drain	560.100yr + 35%	0.23	0.09	11.66	10.85	11.68	0.000834	0.67	4.61	5.23	0.23			
Drain	540.100yr + 35%	0.09	0.16	11.65	10.66	11.67	0.000463	0.54	5.7	5.72	0.17			
Drain	519.9999	100yr + 35%	0.02	0.32	11.64	10.73	11.66	0.000727	0.65	4.75	4.83	0.21		
Drain	514.1835	100yr + 35%	0.11	0.07	11.62	10.81	11.65	0.001169	0.76	4.08	4.64	0.26		
Drain	511.7022	None	Bridge											
Drain	509.4359	100yr + 35%	0.07	0.09	11.54	10.85	11.58	0.001406	0.81	3.82	4.72	0.29		
Drain	499.9999	100yr + 35%	0.17	0.33	11.54	10.71	11.56	0.000668	0.64	4.88	5.17	0.21		
Drain	480.100yr + 35%	0.11	0.23	11.54	10.51	11.55	0.000418	0.52	6	6.06	0.17			
Drain	459.9999	100yr + 35%	0.15	0.19	11.52	10.62	11.54	0.000521	0.57	5.42	5.59	0.19		
Drain	439.9999	100yr + 35%	0.01	0.32	11.51	10.57	11.53	0.000534	0.58	5.38	5.46	0.19		
Drain	420.100yr + 35%	0.07	0.12	11.5	10.64	11.52	0.000678	0.62	4.97	5.42	0.21			
Drain	399.9999	100yr + 35%	0.11	0.23	11.48	10.61	11.5	0.000638	0.61	5.06	5.46	0.2		
Drain	379.9999	100yr + 35%	-0.05	0.36	11.49	10.56	11.49	0.000143	0.3	16.48	42.5	0.1		
Drain	359.9999	100yr + 35%	-0.03	0.23	11.49	10.42	11.49	0.000108	0.27	17.85	42.81	0.09		
Drain	339.9999	100yr + 35%	-0.03	0.16	11.48	10.49	11.49	0.000164	0.33	15.07	42.88	0.1		
Drain	319.9999	100yr + 35%	0	0.01	11.47	10.5	11.48	0.000521	0.53	5.83	6.95	0.19		
Drain	299.9999	100yr + 35%	0.04	0.11	11.46	10.55	11.47	0.000535	0.55	5.64	6.53	0.19		
Drain	279.9999	100yr + 35%	-0.08	0.13	11.46	10.52	11.46	0.000136	0.28	16.93	42.67	0.1		
Drain	260.100yr + 35%	-0.13	0.03	11.46	10.52	11.46	0.000153	0.29	16.21	42.91	0.1			
Drain	240.100yr + 35%	0.16	0.29	11.43	10.6	11.45	0.000581	0.6	5.2	5.53	0.2			
Drain	220.100yr + 35%	0.2	0.2	11.41	10.71	11.43	0.00124	0.7	4.4	6.71	0.28			
Drain	200.100yr + 35%	0.06	0.06	11.4	10.58	11.42	0.000573	0.58	5.31	5.79	0.19			
Drain	179.9999	100yr + 35%	0.27	0.29	11.36	10.77	11.4	0.001388	0.83	3.74	4.6	0.29		
Drain	176.7426	100yr + 35%	0.43	0.43	11.35	10.78	11.39	0.001611	0.86	3.59	4.7	0.32		
Drain	173.7661	None	Bridge											
Drain	170.8786	100yr + 35%	0.48	0.43	11.33	10.74	11.37	0.001477	0.86	3.62	4.37	0.3		
Drain	159.9999	100yr + 35%	0.28	0.36	11.32	10.61	11.35	0.0008	0.65	4.79	5.95	0.23		
Drain	139.9999	100yr + 35%	0.08	0.08	11.32	10.59	11.33	0.000568	0.47	6.54	10.57	0.19		
Drain	120.100yr + 35%	0.15	0.09	11.31	10.55	11.32	0.000476	0.45	6.93	10.82	0.18			
Drain	100.100yr + 35%	-0.08	0.15	11.31	10.67	11.31	0.000218	0.3	14.17	40.74	0.12			
Drain	79.9999	100yr + 35%	0.13	0.03	11.3	10.58	11.3	0.000372	0.41	7.61	11.51	0.16		
Drain	60.100yr + 35%	-0.01	0.04	11.29	10.55	11.3	0.000443	0.44	7	10.7	0.17			
Drain	40.100yr + 35%	0.05	0.1	11.28	10.27	11.29	0.000211	0.34	9	11.29	0.12			
Drain	20.100yr + 35%	0.01	0.09	11.27	10.43	11.28	0.000678	0.48	6.5	11.86	0.21			
Drain	19.05487	None	Culvert											
Drain	12.28586	100yr + 35%	0.26	0.31	11.02	10.29	11.03	0.000636	0.52	6.02	9.38	0.21		
Drain	7.983860	None	Bridge											
Drain	4.517827	100yr + 35%	0.26	0.38	10.99	10.36	11.01	0.001002	0.66	4.68	6.86	0.26		

Table 4.4:1 in 100 Year + 35% Climate Change Plus 25% Increase on Peak Flows

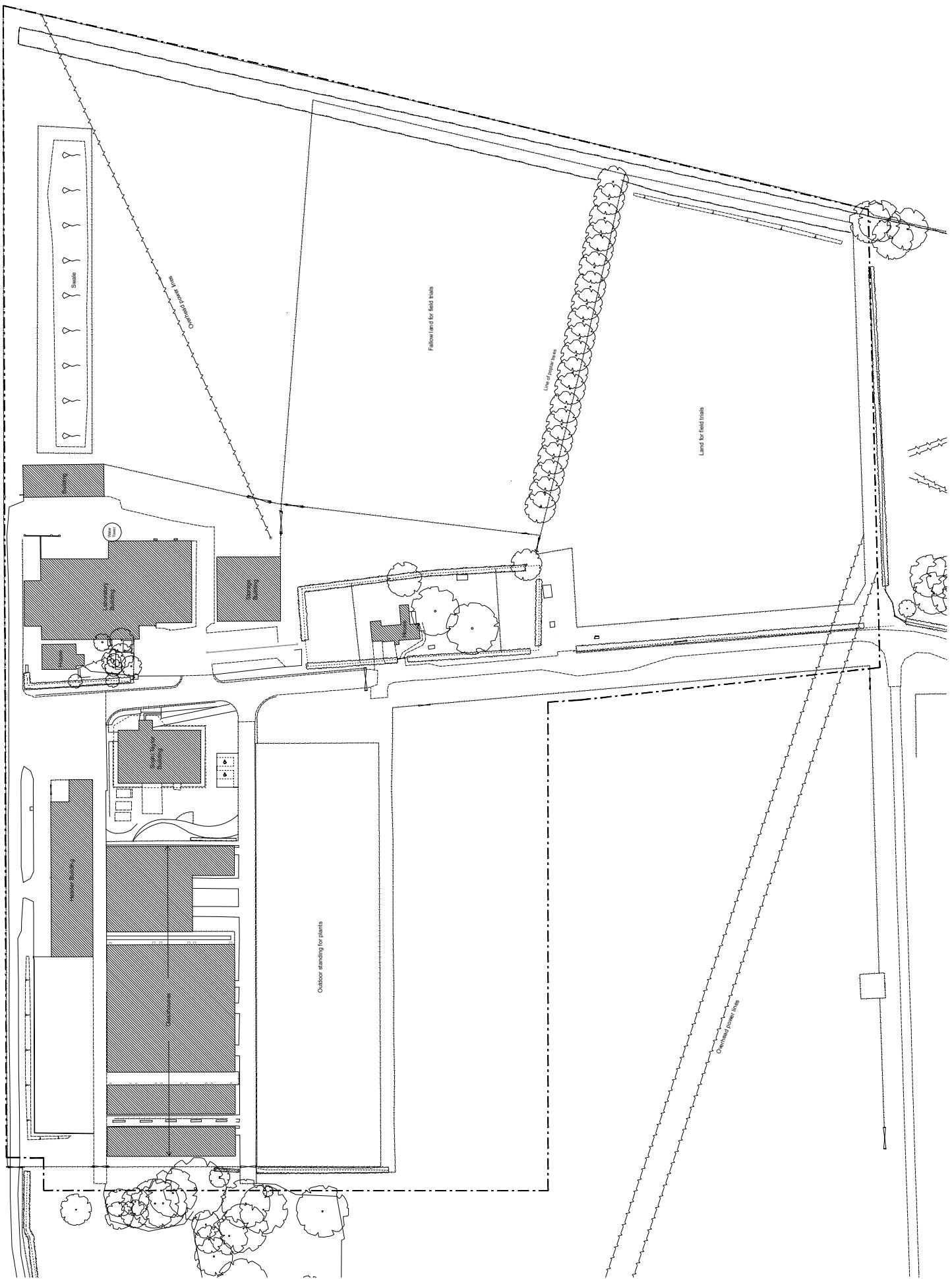
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BFIHOST		0.542				
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DPSBAR		7				
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RMED-1D		28.7				
RMED-2D		34.6				
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SAAR4170		553				
SPRHOST		43.27				
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URBCONC2000		0.771				
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D1		0.31487				
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D3		0.2844				
E		0.31745				
F		2.44653				
C(1 km)		-0.026				
D1(1 km)		0.316				
D2(1 km)		0.248				
D3(1 km)		0.279				
E(1 km)		0.318				
F(1 km)		2.442				

**PLANS**



Planning Application Site Boundary



1 - 25.11.2016  
Ref: 00-101  
Rev Date: 10/11/16  
Cust. Drawn

PS UB

00-101  
Cust. Drawn

05/27  
1:500  
A1  
UB

smith | evans  
architects

Project: NLRB Cambridge  
Drawing: Park Farm

Site Plan

Existing

Job Ref:

Drawing No. 00-101

Revision 1

Date: Drawn

Scale: 1:500

Sheet: A1

Page: 1

Revised:

Comments:

Planning





Comments	Created By	Date	PEV	Scope at A1
Project Number				
2219-545	Custom			



**1 Chestnut Place, Cringlford  
Norwich, Norfolk NR4 7BD**

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Edward Fenzl

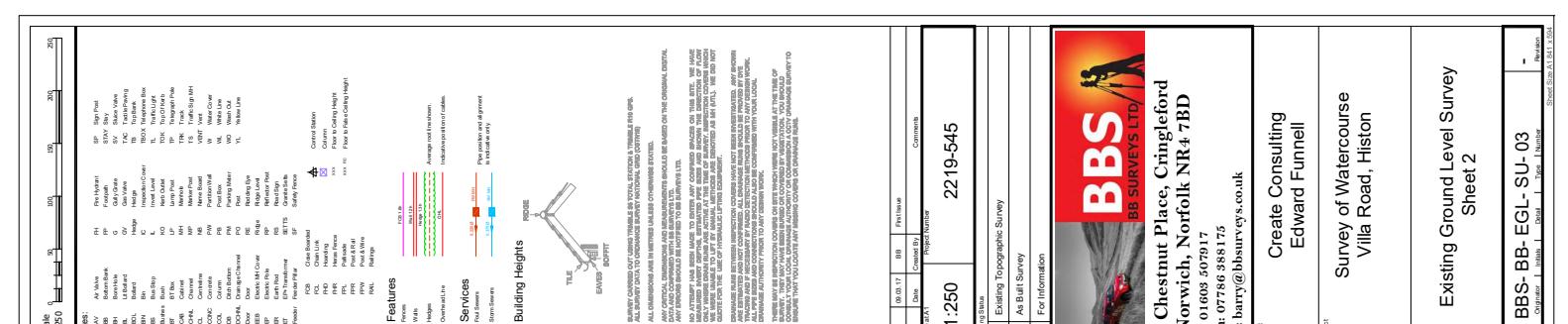
Survey of Watercourse  
Villa Road Histon

Existing Ground Level Survey

BBS- BB- EGL- SU- 01 - Revision  
Schem S220 A1 841 x 559







REV	Date	By	Comments
1.250	22/19-545		

X Existing Topographic Survey  
As Built Survey  
For Information



1 Chestnut Place, Cringleford  
Norwich, Norfolk NR4 7BD

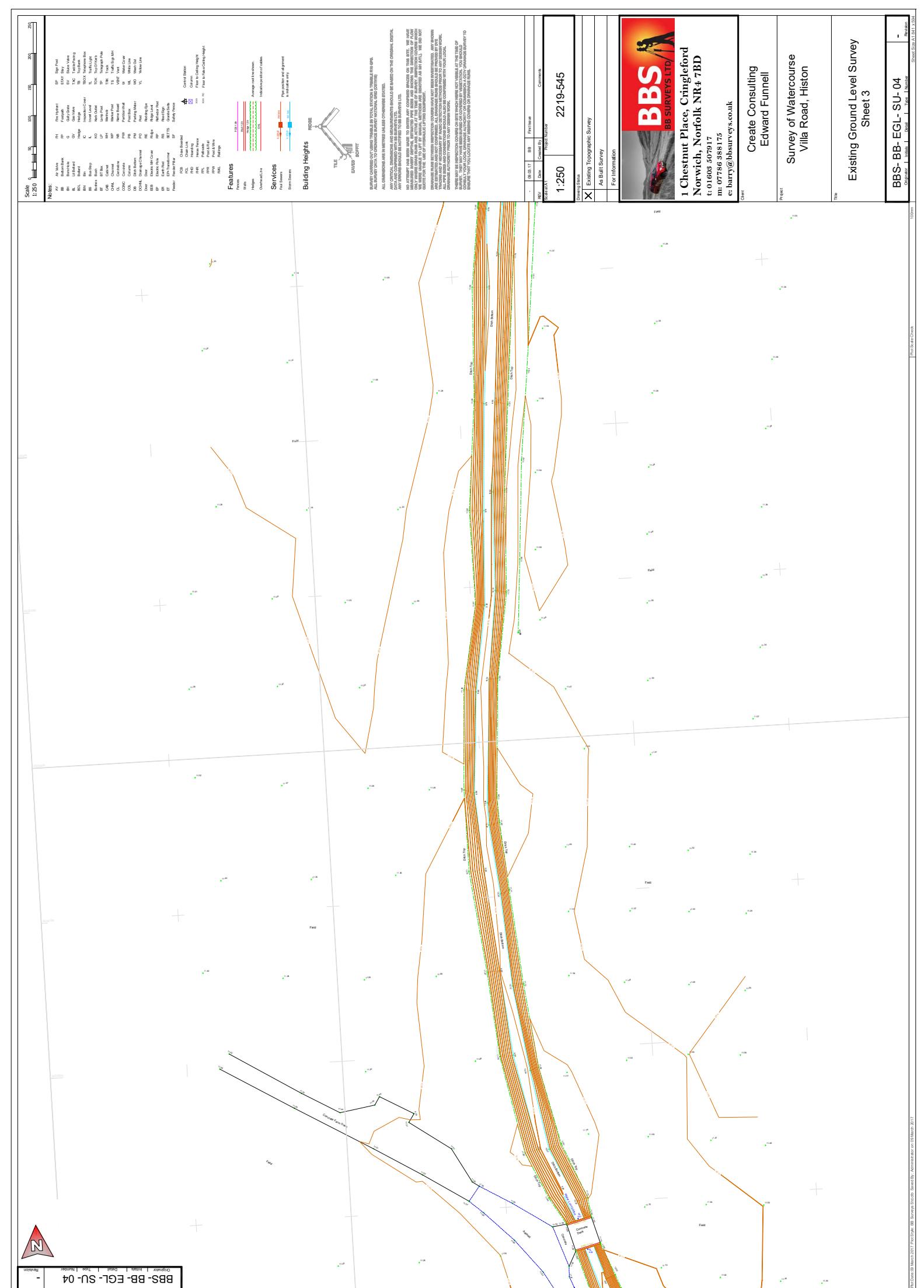
Mr. 01603 507917  
e: harry@bbsurveys.co.uk

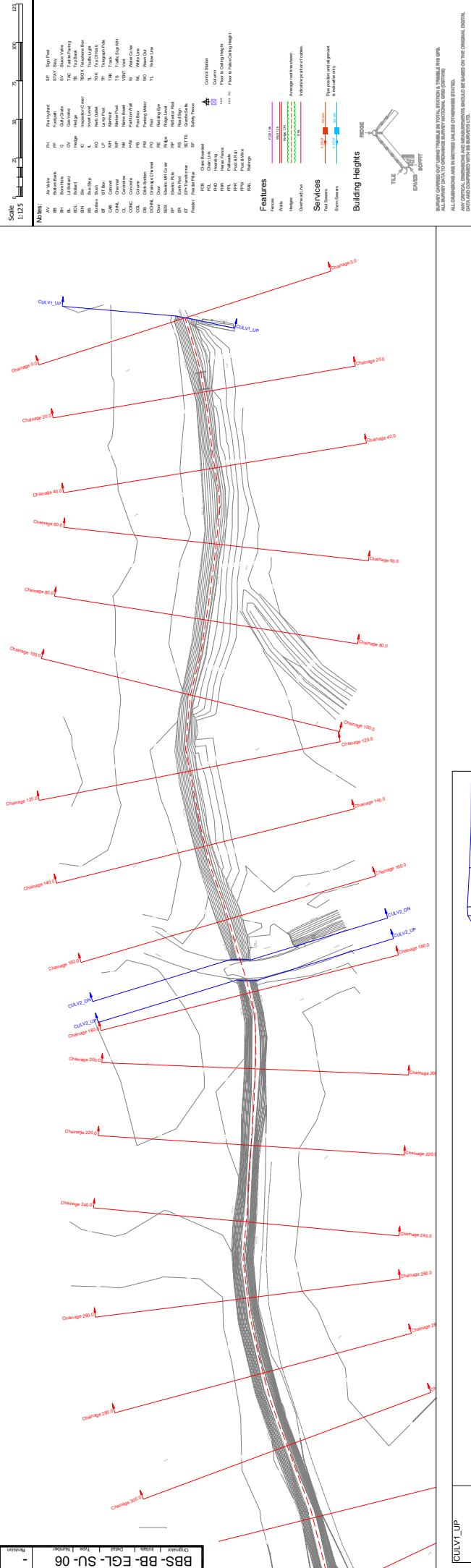
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Survey of Watercourse  
Villa Road, Histon

Existing Ground Level Survey  
Sheet 2

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Project: [redacted] Job No: [redacted]  
Scale: 1:250 Date: [redacted]





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09/03/17	08	Comments																							
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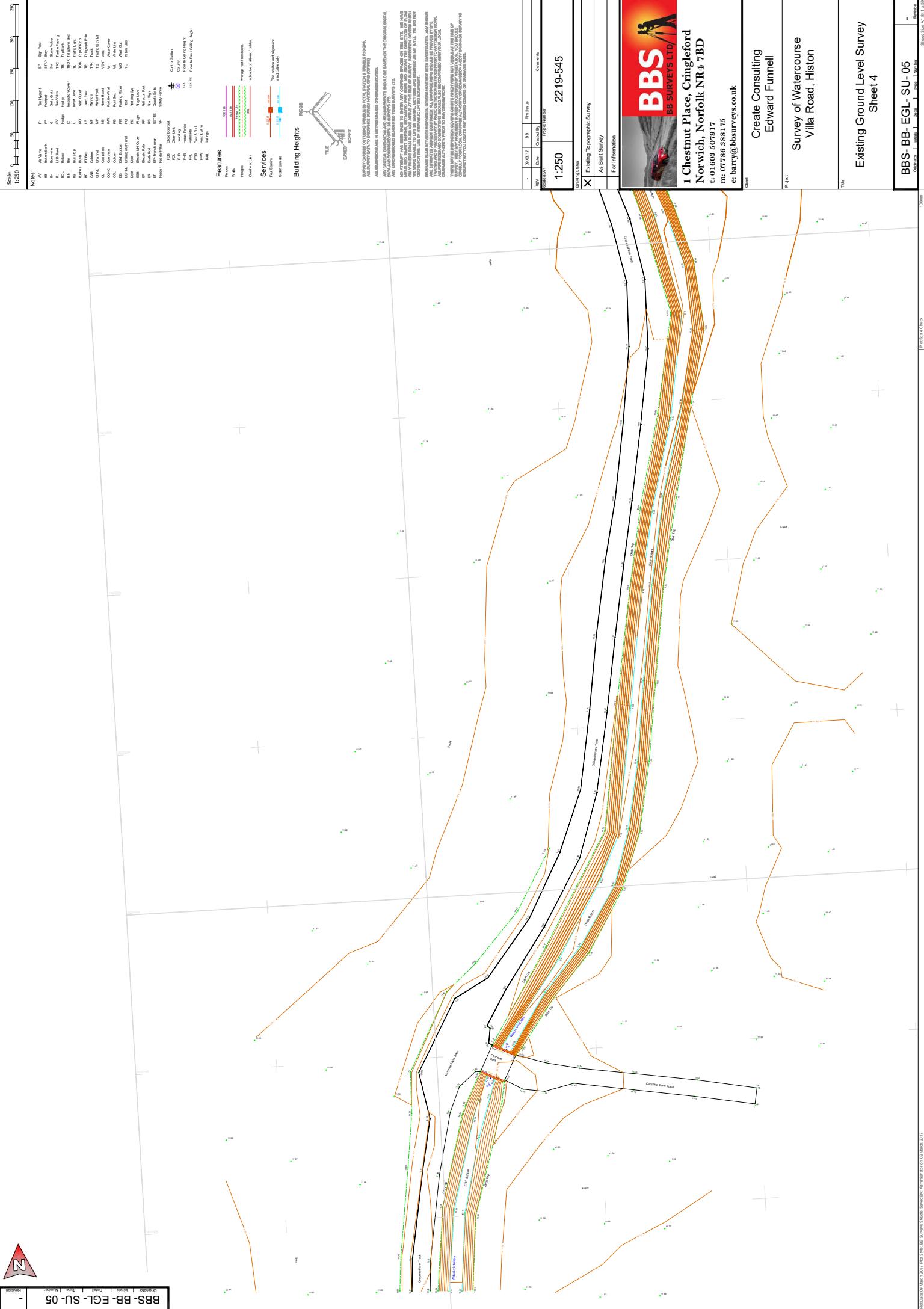


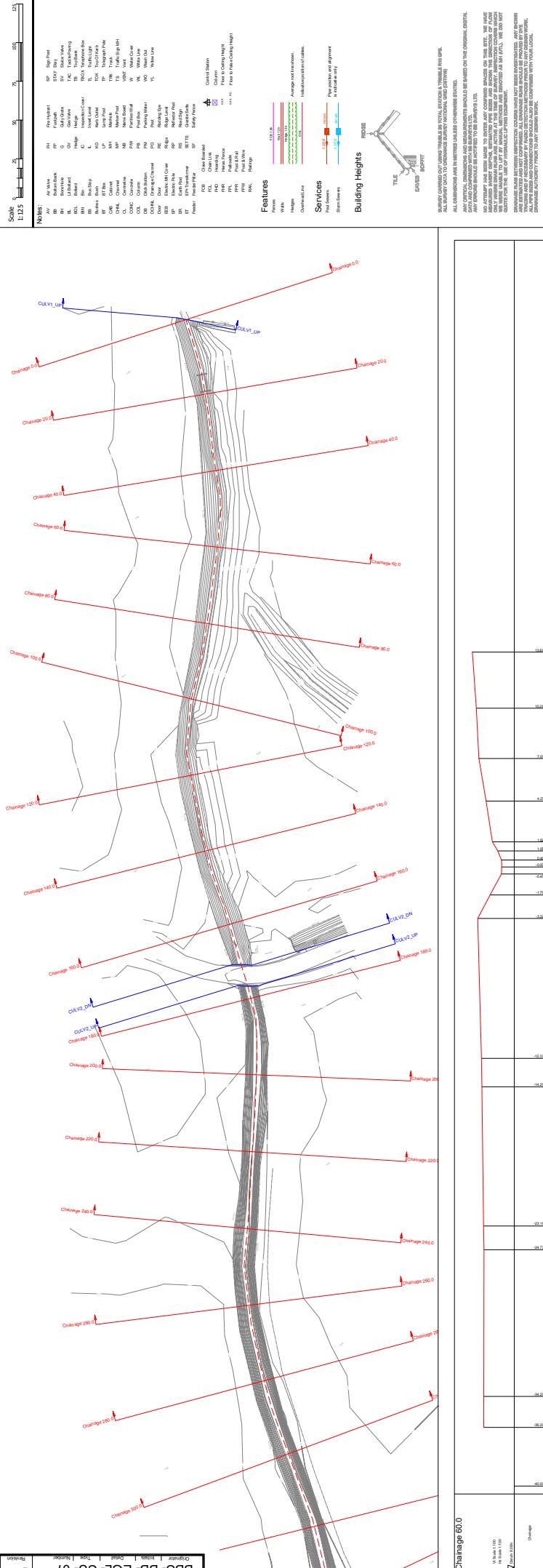
1 Chestnut Place, Cringleford  
Norwich, Norfolk NR4 7BD  
t: 01603 507917

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Survey of Watercourse  
Villa Road, Histon

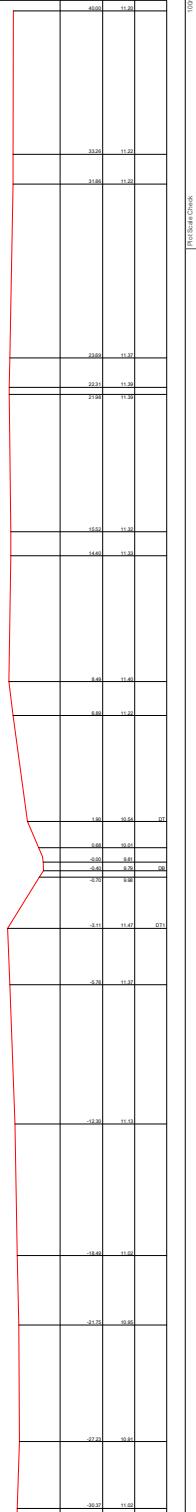
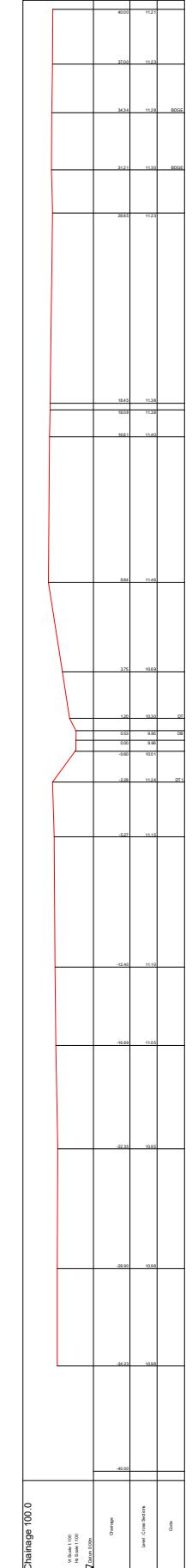
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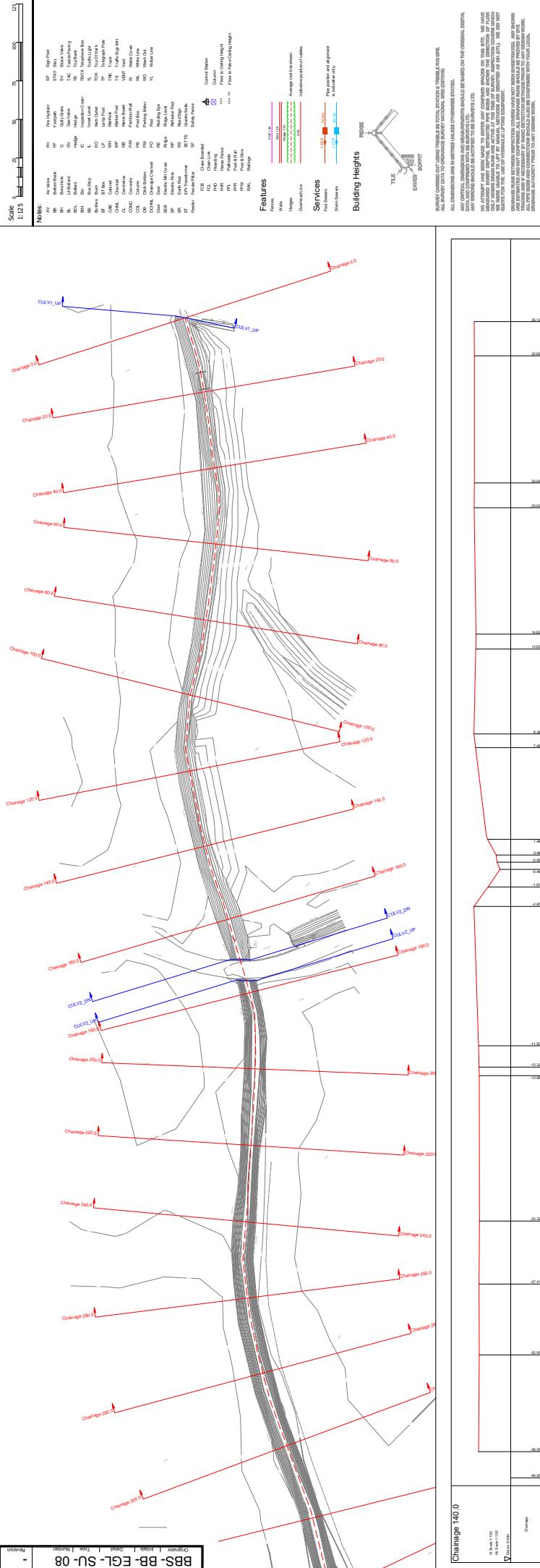




**1 Chestnut Place, Cringleford  
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t: 01603 057917  
m: 07786 388175  
e: [bhsurveys.co.uk](mailto:bhsurveys.co.uk)

Project	Survey of Watercourse Villa Road, Histon		
Time			
BBS - BB- EGL- SU- 07	Cross Section CH60 - CH120		
Comments	Survey Area A 541.05.05.0000 Survey Date 05/05/2005 Surveyor Name		





**Change 160.0**

Legend:

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- Streams (100)
- Property Lines (100)
- Survey Line (100)

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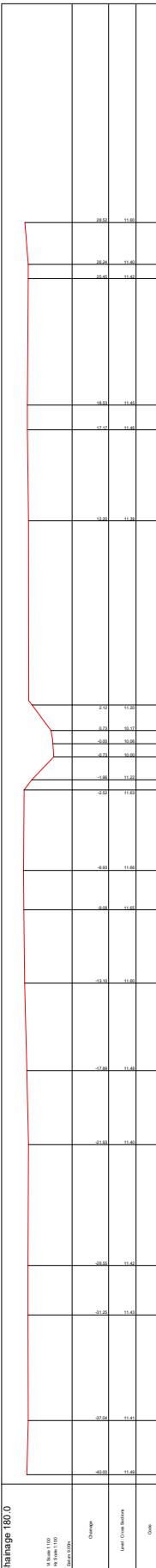
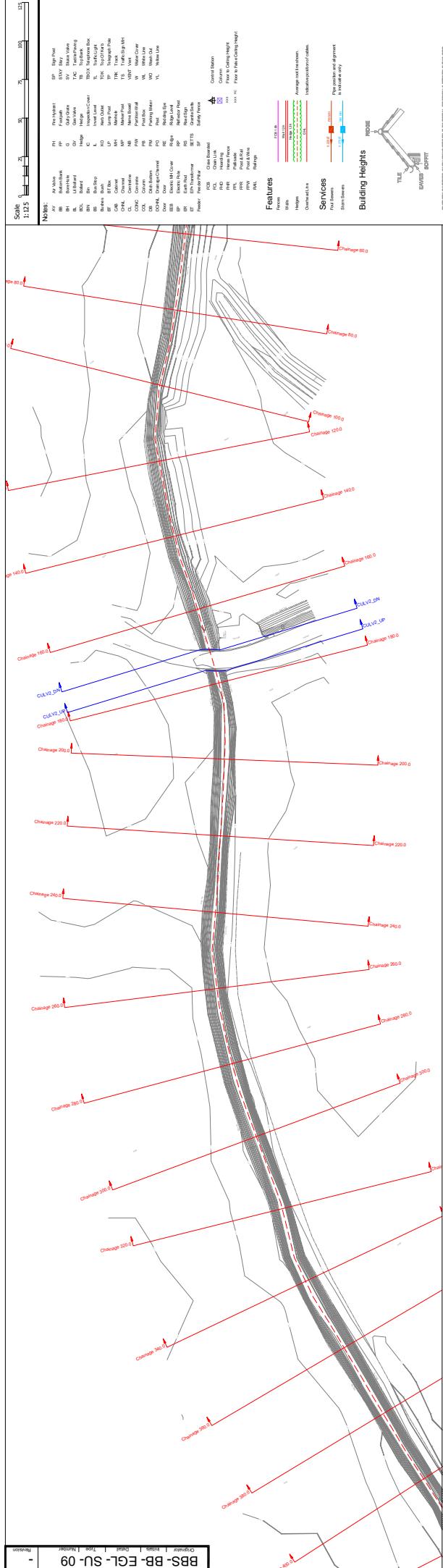
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		Existing Topographic Survey
		As Built Survey
		For Information



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Survey of Watercourse  
Vol. 2



	(00:03:17)	BB	FMS Issue	Comments
Rev.	Date	Created By	Project Number	
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1 Chestnut Place, Cringleford  
Norwich, Norfolk NR4 7BD

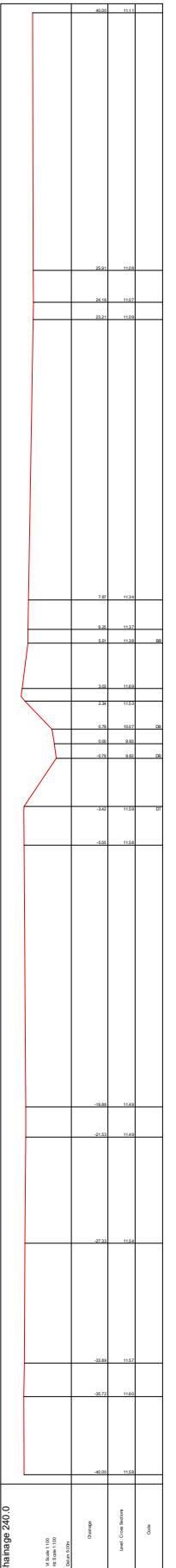
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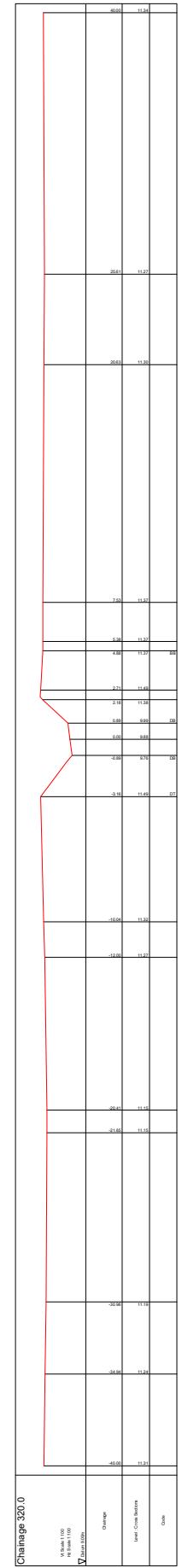
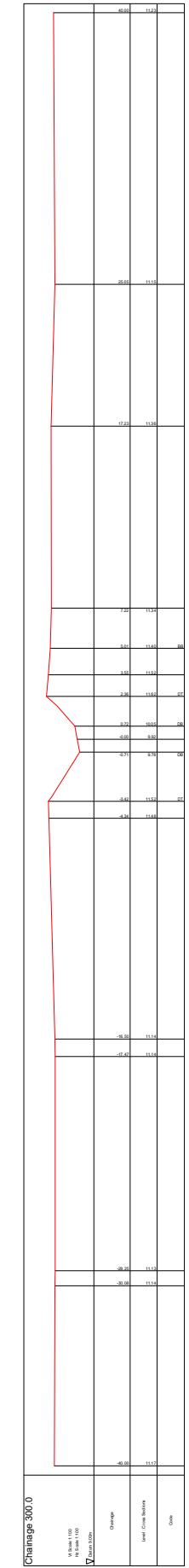
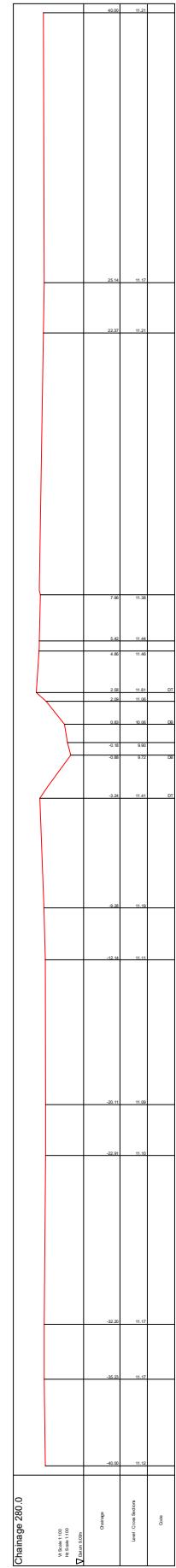
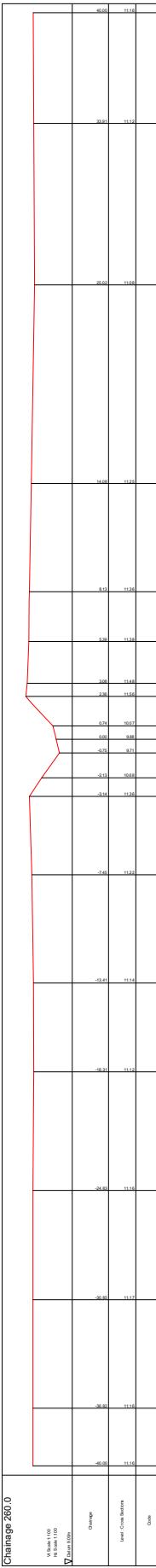
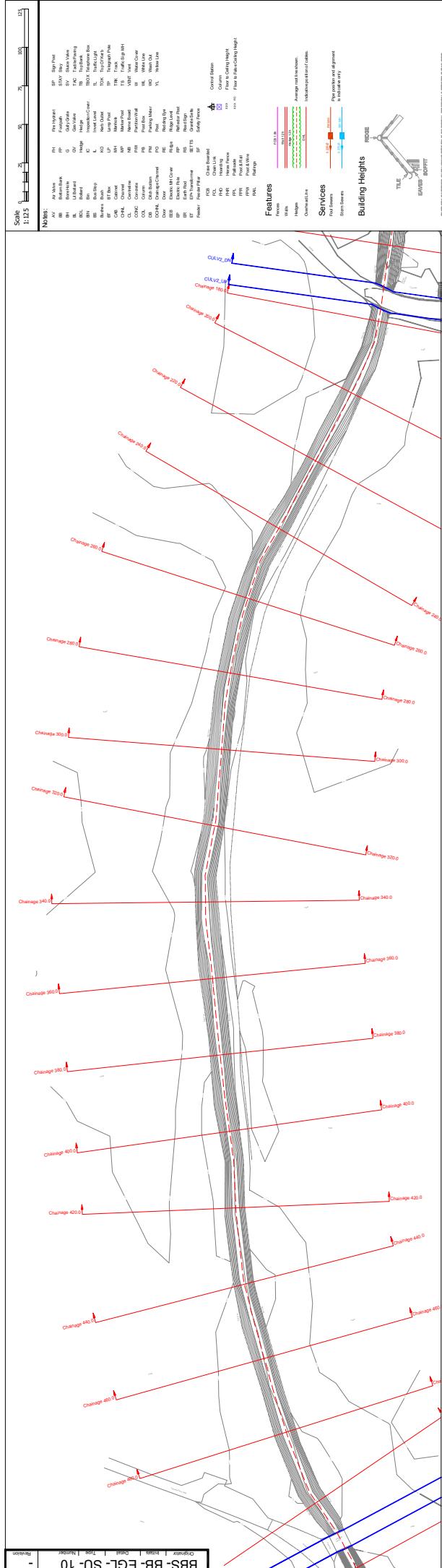
**Survey of Watercourse  
Villa Road, Histon**

Survey of watercourse  
Villa Road, Histon

Cross Sections  
CH180 - CH240

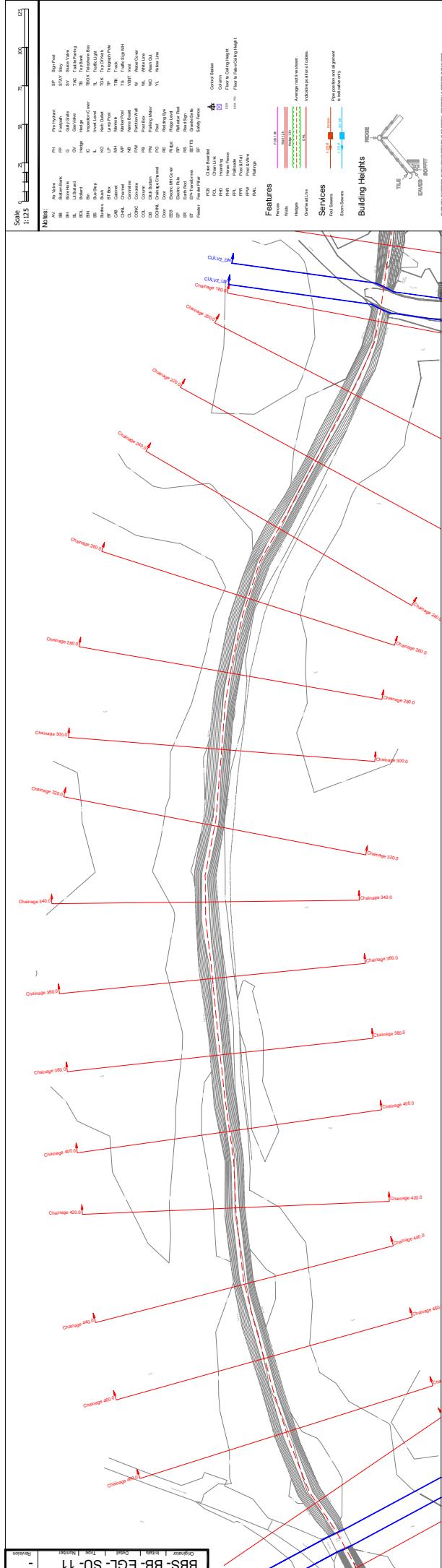
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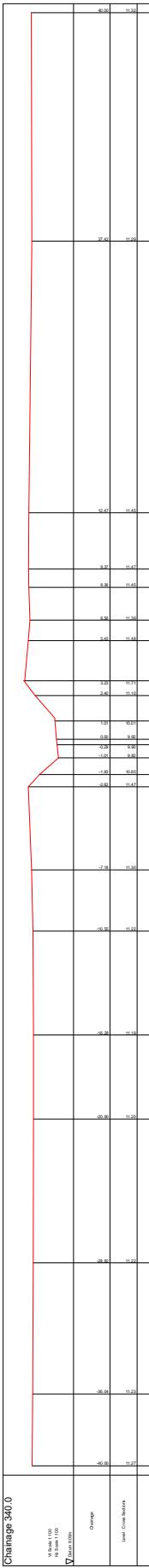
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Page 12 of 14 | World Bank Create Consulting LLC - Create Consulting - Survey



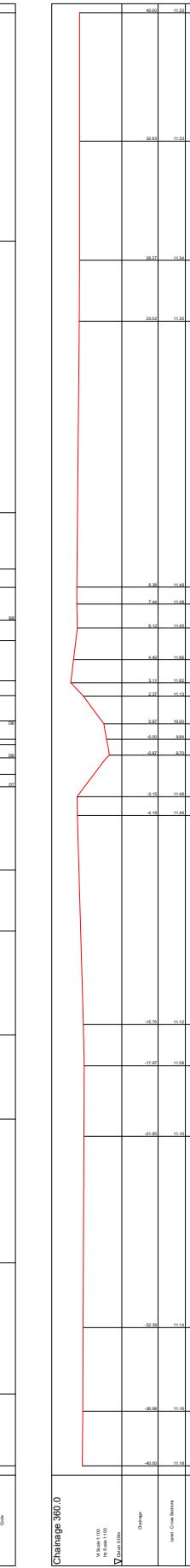
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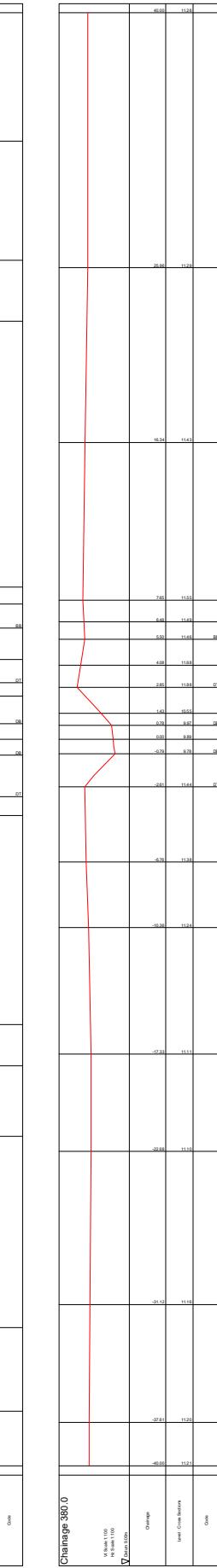
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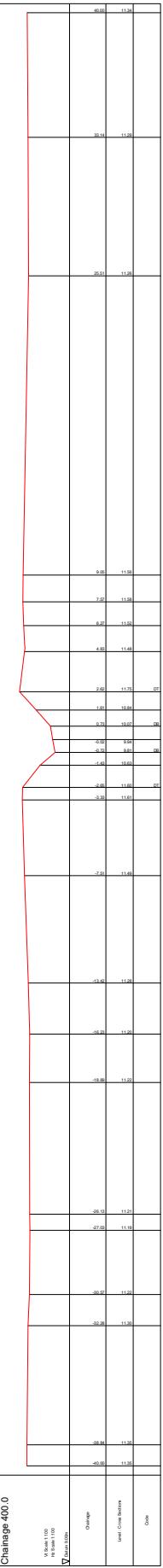
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[View Details](#)



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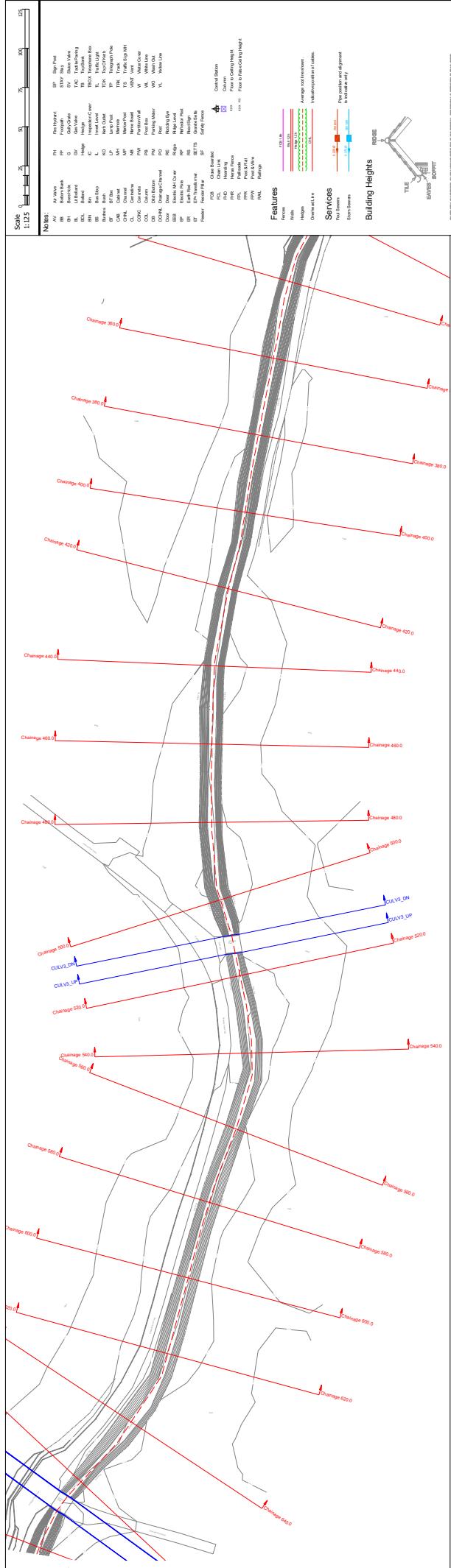
Survey of Watercourse  
Villa Road, Histon

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Title

Cross Sections  
CH340 - CH400

BBS- BB- EGI - SII- 11 -



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Image 480\_0

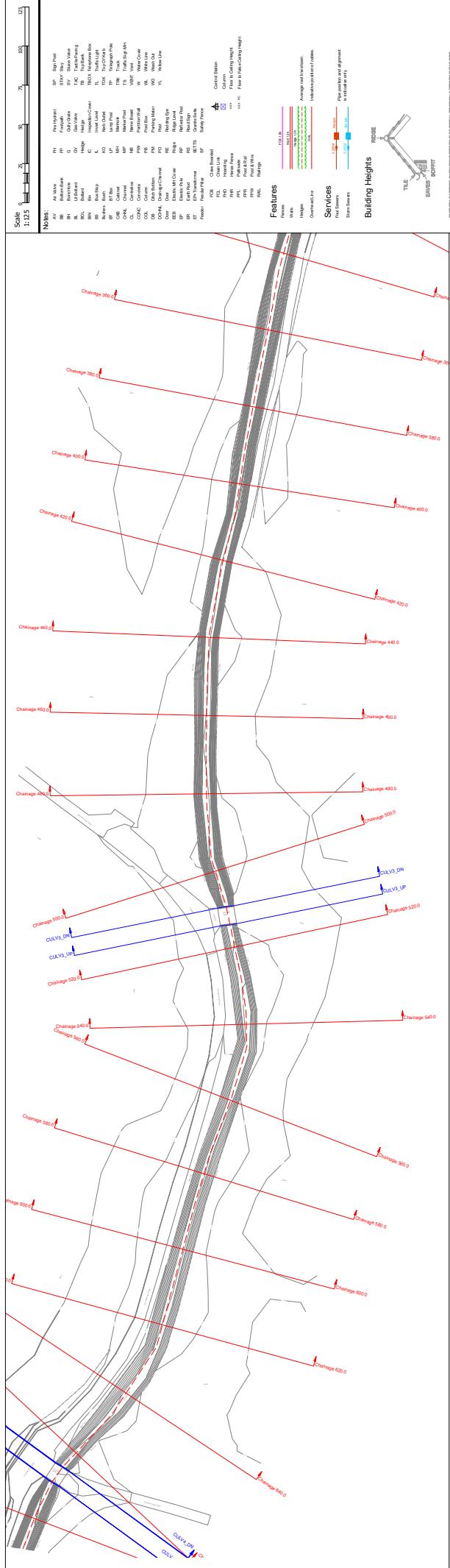
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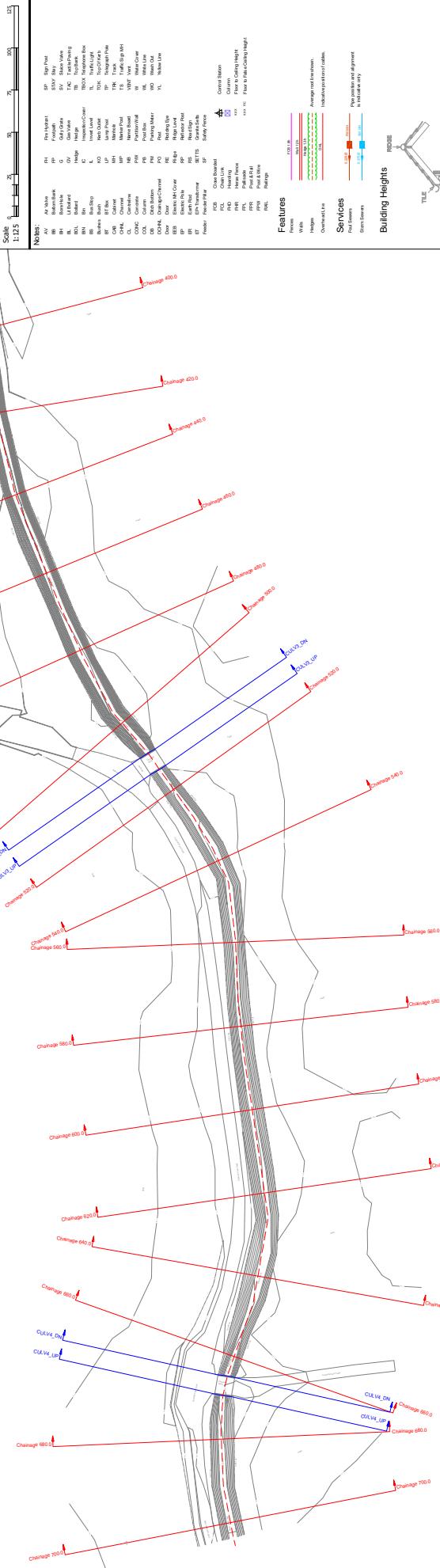
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-30.00 to -29.00	~100
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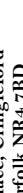


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Changeage 280.0



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0778698817

arry@bbsurveys.co.uk

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CH540 - C

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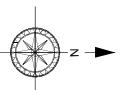
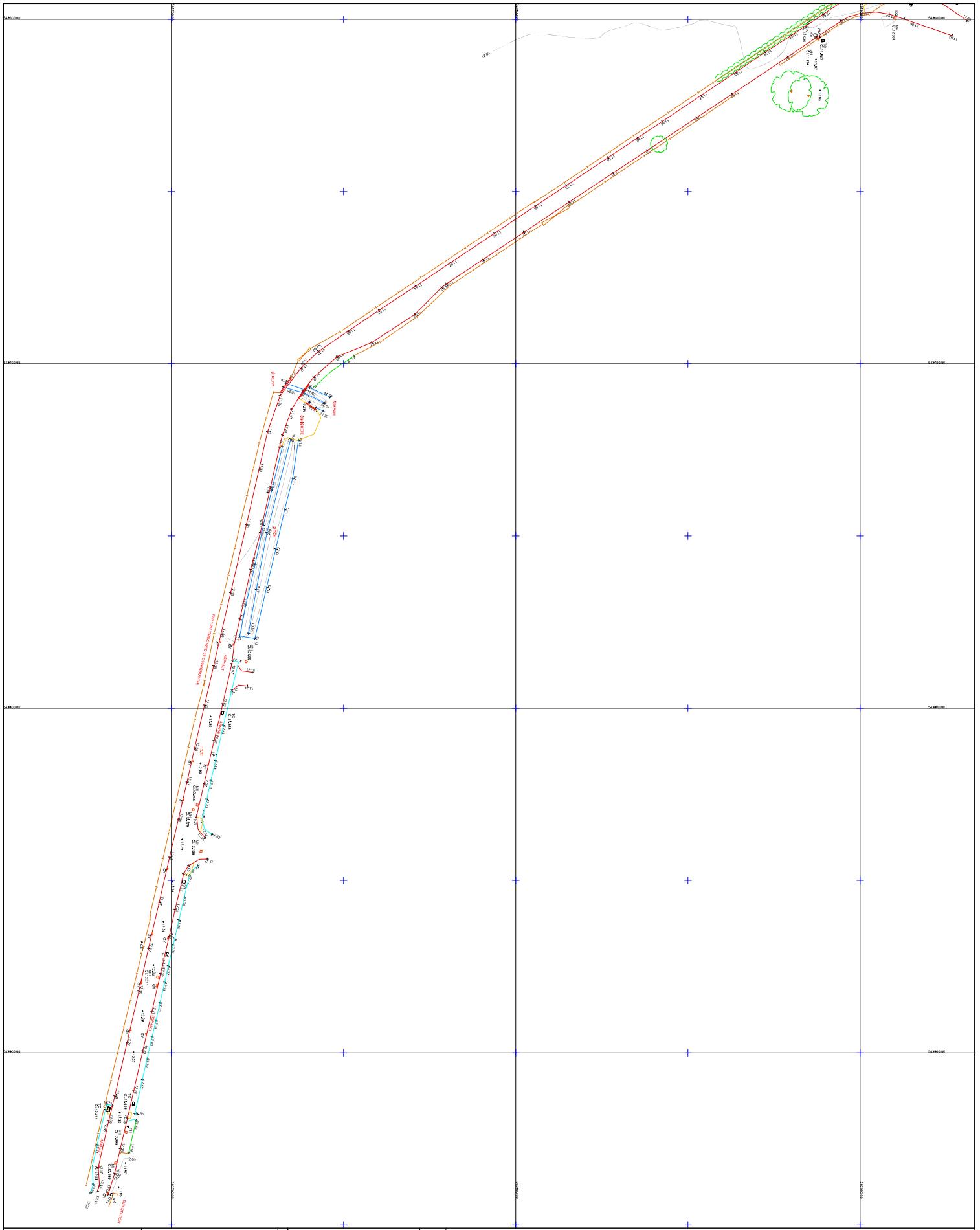
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J68	J78	543499.95	26374.81
J69	J79	543499.99	26381.48
J70	J80	543499.03	26388.15
J71	J81	543499.07	26394.82
J72	J82	543499.11	26401.49
J73	J83	543499.15	26408.16
J74	J84	543499.19	26414.83
J75	J85	543499.23	26421.50
J76	J86	543499.27	26428.17
J77	J87	543499.31	26434.84
J78	J88	543499.35	26441.51
J79	J89	543499.39	26448.18
J80	J90	543499.43	26454.85
J81	J91	543499.47	26461.52
J82	J92	543499.51	26468.19
J83	J93	543499.55	26474.86
J84	J94	543499.59	26481.53
J85	J95	543499.63	26488.20
J86	J96	543499.67	26494.87
J87	J97	543499.71	26501.54
J88	J98	543499.75	26508.21
J89	J99	543499.79	26514.88
J90	J100	543499.83	26521.55

1

1000

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G.L. Chaitin-Golob / Proc. R. Soc. Lond. A 455 (1999) 231–244

CB Cash Box  
CH Chimey  
G1 Overhead  
GDN Distance Bench

DR Dan  
ELC Eleggy

卷之三

Date: October 2016

Drawing Number: ALS75618/500/62  
Instrument

104

Villa Rossi, Históri

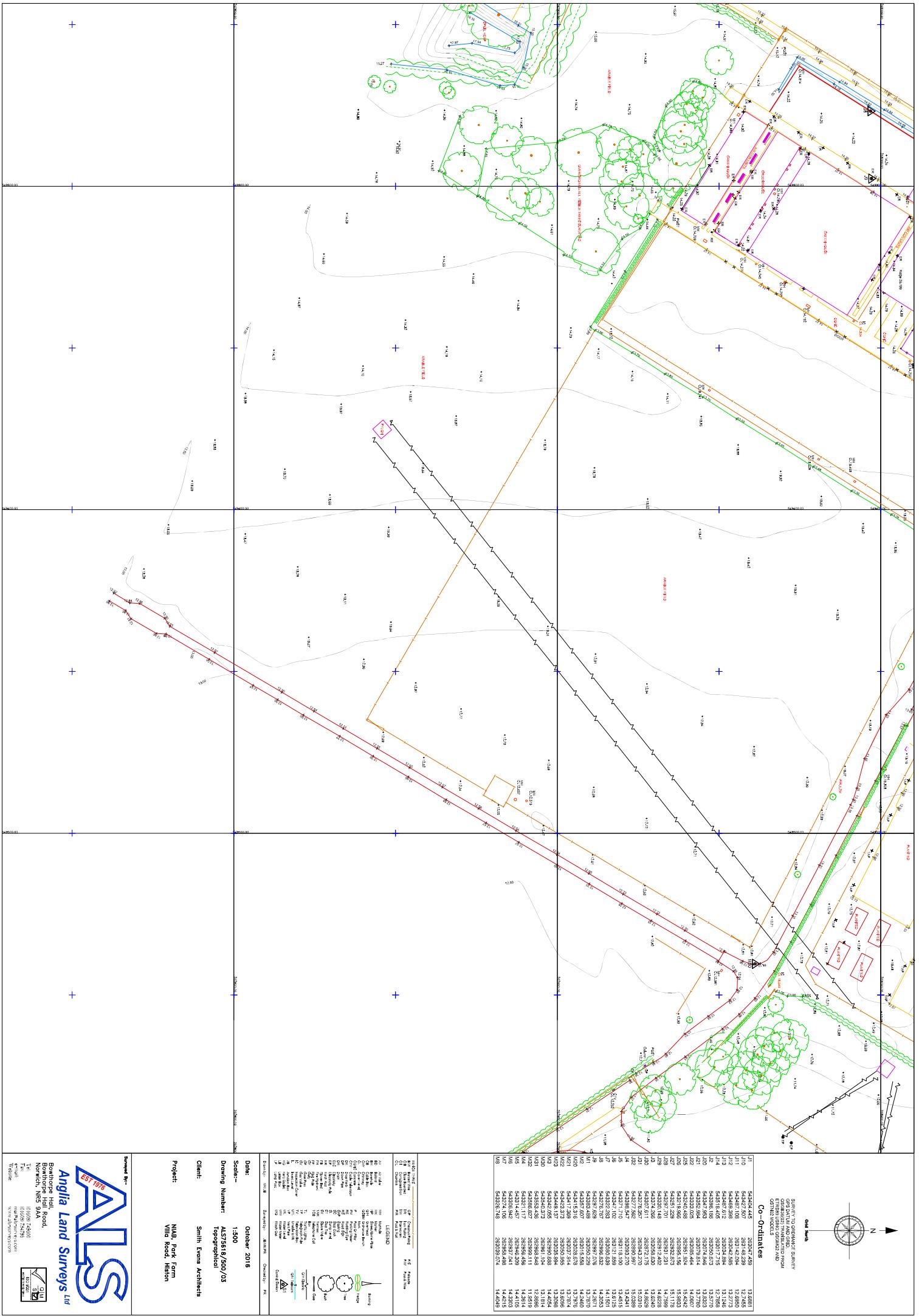
11 of 11

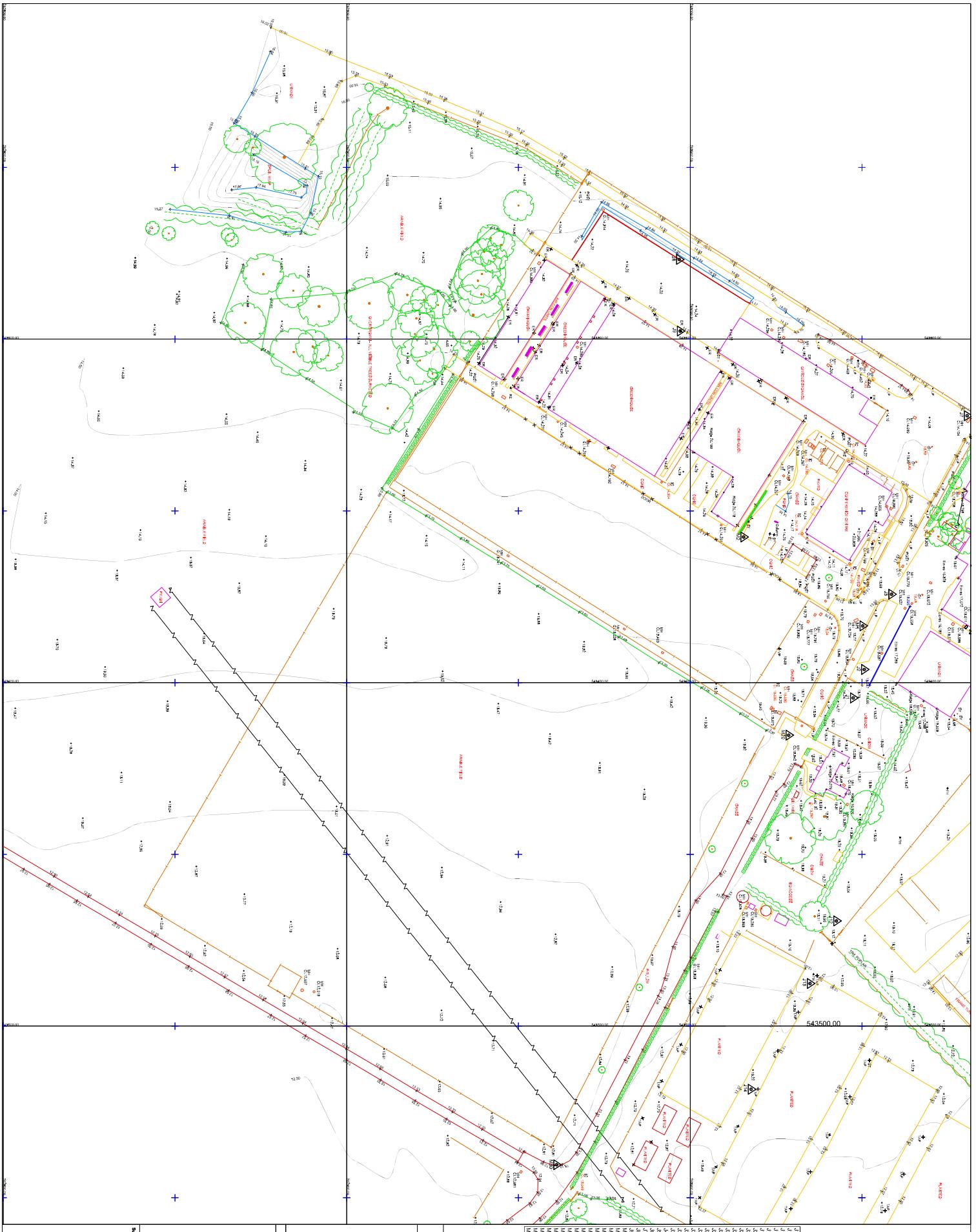
76

D  
E  
C

Bowthorpe Hall,  
Bowthorpe Hall,  
Road.

01699-38738  
marketsurveys.com





Surveyed By:		Date:	October 2016
Scales:-	1:500	Drawing Number:	ALST518/500/04
Client:	Smith Evans Architects	Project:	NIB, Park Farm Villa Road, Histon
Instrument:	Topographical	Surveyor:	Anglia Land Surveys Ltd
Surveyor:	Anglia Land Surveys Ltd	Surveyor No.:	ALS
Surveyor Address:	Bottesford Hall Road, Norwich NR5 9AA	Surveyor Tel. No.:	01603 746200
Surveyor Email:	info@als.surveys.com	Surveyor Fax No.:	01603 746275
Surveyor Web:	www.als.surveys.com	Surveyor Licence No.:	ALS0012



Survey to Ordnance Survey  
Grid References Translated from  
Easting and Northing and  
Given in Grid References

#### Co-ordinates

	Co-ordinates
J1	543420.445 202847.439
J10	543420.7307 202848.239
J11	543420.8456 202848.436
J12	543420.8556 202848.535
J13	543420.8588 202848.592
J14	543420.8605 202848.633
J15	543420.8635 202848.670
J16	543420.8665 202848.700
J17	543420.8695 202848.729
J18	543420.8725 202848.758
J19	543420.8755 202848.787
J20	543420.8785 202848.816
J21	543420.8815 202848.844
J22	543420.8845 202848.873
J23	543420.8875 202848.902
J24	543420.8905 202848.931
J25	543420.8935 202848.960
J26	543420.8965 202848.989
J27	543420.8995 202849.018
J28	543420.9025 202849.047
J29	543420.9055 202849.076
J30	543420.9085 202849.105
J31	543420.9115 202849.134
J32	543420.9145 202849.163
J33	543420.9175 202849.192
J34	543420.9205 202849.221
J35	543420.9235 202849.250
J36	543420.9265 202849.279
J37	543420.9295 202849.308
J38	543420.9325 202849.337
J39	543420.9355 202849.366
J40	543420.9385 202849.395
J41	543420.9415 202849.424
J42	543420.9445 202849.453
J43	543420.9475 202849.482
J44	543420.9505 202849.511
J45	543420.9535 202849.540
J46	543420.9565 202849.569
J47	543420.9595 202849.598
J48	543420.9625 202849.627
J49	543420.9655 202849.656
J50	543420.9685 202849.685
J51	543420.9715 202849.714
J52	543420.9745 202849.743
J53	543420.9775 202849.772
J54	543420.9805 202849.801
J55	543420.9835 202849.830
J56	543420.9865 202849.859
J57	543420.9895 202849.888
J58	543420.9925 202849.917
J59	543420.9955 202849.946
J60	543420.9985 202849.975
M1	543420.9985 202850.004
M2	543420.9955 202850.033
M3	543420.9925 202850.062
M4	543420.9895 202850.091
M5	543420.9865 202850.120
M6	543420.9835 202850.149
M7	543420.9805 202850.178
M8	543420.9775 202850.207
M9	543420.9745 202850.236
M10	543420.9715 202850.265
M11	543420.9685 202850.294
M12	543420.9655 202850.323
M13	543420.9625 202850.352
M14	543420.9595 202850.381
M15	543420.9565 202850.410
M16	543420.9535 202850.439
M17	543420.9505 202850.468
M18	543420.9475 202850.497
M19	543420.9445 202850.526
M20	543420.9415 202850.555
M21	543420.9385 202850.584
M22	543420.9355 202850.613
M23	543420.9325 202850.642
M24	543420.9295 202850.671
M25	543420.9265 202850.700
M26	543420.9235 202850.729
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M28	543420.9175 202850.787
M29	543420.9145 202850.816
M30	543420.9115 202850.845
M31	543420.9085 202850.874
M32	543420.9055 202850.903
M33	543420.9025 202850.932
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M35	543420.9065 202850.990
M36	543420.9035 202851.019
M37	543420.9005 202851.048
M38	543420.8975 202851.077
M39	543420.8945 202851.106
M40	543420.8915 202851.135
M41	543420.8885 202851.164
M42	543420.8855 202851.193
M43	543420.8825 202851.222
M44	543420.8795 202851.251
M45	543420.8765 202851.280
M46	543420.8735 202851.309
M47	543420.8705 202851.338
M48	543420.8675 202851.367
M49	543420.8645 202851.396
M50	543420.8615 202851.425

Feature No.	Area - acres	Comments
1	0.0000	Ordnance Survey Grid Reference
2	0.0000	Ordnance Survey Grid Reference
3	0.0000	Ordnance Survey Grid Reference
4	0.0000	Ordnance Survey Grid Reference
5	0.0000	Ordnance Survey Grid Reference
6	0.0000	Ordnance Survey Grid Reference
7	0.0000	Ordnance Survey Grid Reference
8	0.0000	Ordnance Survey Grid Reference
9	0.0000	Ordnance Survey Grid Reference
10	0.0000	Ordnance Survey Grid Reference
11	0.0000	Ordnance Survey Grid Reference
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14	0.0000	Ordnance Survey Grid Reference
15	0.0000	Ordnance Survey Grid Reference
16	0.0000	Ordnance Survey Grid Reference
17	0.0000	Ordnance Survey Grid Reference
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20	0.0000	Ordnance Survey Grid Reference
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26	0.0000	Ordnance Survey Grid Reference
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30	0.0000	Ordnance Survey Grid Reference
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32	0.0000	Ordnance Survey Grid Reference
33	0.0000	Ordnance Survey Grid Reference
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43	0.0000	Ordnance Survey Grid Reference
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45	0.0000	Ordnance Survey Grid Reference
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47	0.0000	Ordnance Survey Grid Reference
48	0.0000	Ordnance Survey Grid Reference
49	0.0000	Ordnance Survey Grid Reference
50	0.0000	Ordnance Survey Grid Reference

Surveyed By:

**Anglia Land Surveys Ltd**  
Established 1978

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Project: NBB Park Farm  
Villa Road, Histon